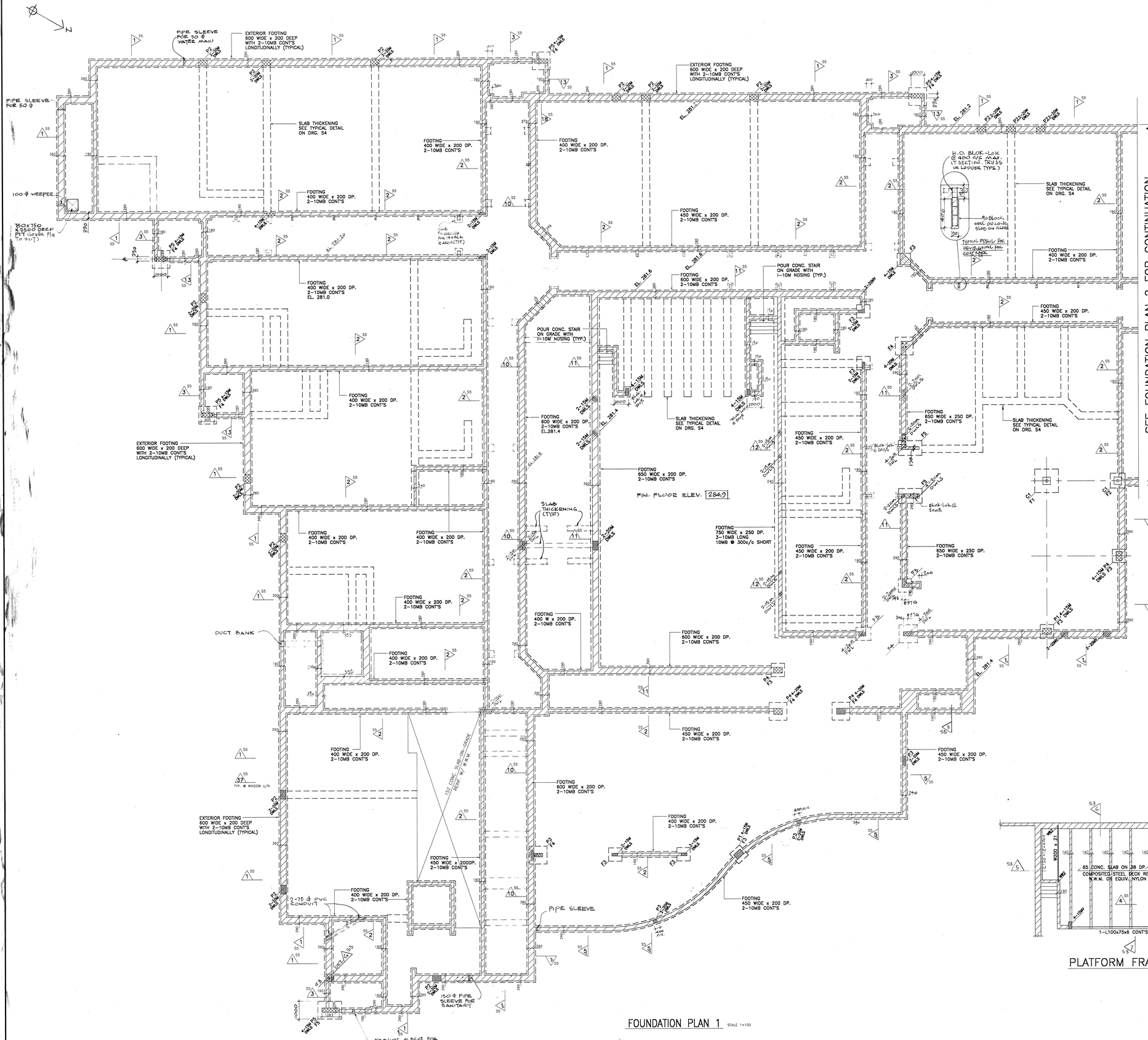


FOUNDATION PLAN 1 SCALE 1=100

PLATFORM FRAMING PLAN SCALE 1=100

FOUNDATION PLAN NOTES :

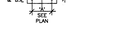
1. FOOTING TO BEAR ON SOIL CAPABLE OF SUSTAINING A LOAD OF 200 KPa EXTERIOR FOOTINGS TO BE A MIN. OF 1500mm BELOW FINISHED GRADE
2. REFER TO SOIL REPORT PREPARED BY PETO MacCALLUM LTD REF. 92BF056 FOR SPECIFICATIONS AND OTHER RECOMMENDATIONS.
3. THE LINE OF SLOPE BETWEEN ADJACENT OR STEPPED FOOTINGS TO BE A MAX. OF 7 IN 10, STEPS TO BE 600 MAXIMUM.
4. CONCRETE STRENGTH : FOR FOOTINGS : 20 MPa AT 28 DAYS. SLAB-ON-GRADE : 25 MPa AT 28 DAYS
5. ALL CONCRETE STRENGTHS SPECIFIED ON PLAN ARE FOR STRUCTURAL REQUIREMENTS ONLY. CONCRETE MIX TO BE DESIGNED WITH A MAXIMUM PERMISSIBLE CEMENT-WATER RATIO IN ACCORDANCE WITH TABLE 7 OF CSA CAN3-A23.1-M90.
6. SLAB-ON-GRADE TO BE 100mm THICK, REINFORCED WITH 1 LAYER 152 x 152 MW 18.7 x 18.7 W.W.MESH, PLACED IN CENTRE OF SLAB. REFER TO ARCHITECTURAL DRG. FOR LOCATIONS OF CONTROL JOINTS IN SLAB-ON GRADE.
7. ALL FOOTINGS, CAPS AND PIERS SHALL BE SYMMETRICAL UNDER COLUMNS AND WALL UNLESS OTHERWISE NOTED.
8. REFER TO DRAWING S1 FOR FOOTING SCHEDULE.
9. REINFORCING STEEL IN FOOTINGS TO BE CSA G30.12 DEFORMED BARS, GRADE 400. Fy = 400 MPa.
10. SOILS ENGINEER TO INSPECT AND APPROVE FOUNDING SOIL PRIOR TO POURING OF CONCRETE FOOTING.
11. REFER TO GENERAL NOTES AND TYPICAL DETAILS ON DRAWING S4.
12. REFER TO OTHER CONSULTANT'S DRAWINGS FOR LOCATIONS OF PITS, DEPRESSIONS IN SLAB-ON-GRADE, ETC. SEE DRAWING S4 FOR TYPICAL DETAIL.
13. ELEVATIONS AT BOTTOM OF FOOTINGS ARE SHOWN ON PLAN THUS EL.
14. S.F. DENOTES STEPPED FOOTINGS.
15. BACKFILL BELOW SLAB-ON-GRADE TO BE GRANULAR MATERIALS COMPACTED TO 98% MODIFIED PROCTOR. RESULTS OF COMPACTION TO BE REPORTED TO THIS OFFICE. ALSO SEE SOIL REPORT FOR RECOMMENDATIONS .
16. PRIOR TO POURING OF FOOTINGS, CHECK LOCATIONS OF DRAINS, PITS, ETC. THAT THEY WILL AFFECT ADJACENT FOOTINGS. LOWER FOOTINGS TO SUIT WHERE REQUIRED.
17. GROUND WATER CONTROL MAY BE REQUIRED IN CONNECTION WITH EXCAVATION FOR FOOTINGS, REFER TO SOIL REPORT FOR RECOMMENDATIONS.
18. PROVIDE MIN. 200 LINTEL BLOCK REINFORCED WITH 2-20M T&B OVER CONDUIT PIPES, ETC. @ FOUNDATION WALL.
19. ALL WALL FOOTING TO HAVE MIN. 100 PROJECTION ON EACH SIDE OF WALL, MIN. 200 DEEP UNLESS OTHERWISE NOTED ON PLAN OR SECTIONS. E.G. W.F. WITH 2-10M @ CONT'S.
20. FOR FACING PURPOSES, EXT. OF FOOTING NOT COVERED BY H.O. BY BOREHOLES ASSUMED TO BE 281.46 U/S ON PLAN.

COLUMN SCHEDULE

MARK	C1	C2			
U/S OF LINTEL LG					
T/STEEL ROOF					
T/CONC. SLAB FIN. MECH. RM.	HSS 203.2x203.2x6.35	HSS 203.2 x 101.6x6.35	50 GROUT	LINTEL	
T/FIN. GRD. FLR.	250			HSS 101.6x101.6x6.35 (AT GRAVITY)	
DISTANCE TO U/S OF BASE PLATE					
COL. BASE PLATE	356x356x25	305x254x20	203x203x12		
NO. OF ANCH. BOLT	4	4	4		

1. ALL ANCHOR BOLTS TO BE DIA. 3/4" x 18" L 3"
2. ALL COLUMN BASE PLATES TO BE BEAR ON 2" GROUT (4000 PSI AT 28 DAYS)
3. COLUMN SHOWN ON SCHEDULE ARE DIAGRAMMATIC ONLY SEE FRAMING PLAN & SECTIONS FOR CORRECT LENGTH OF COLUMNS.

COLUMN	'A'
HSS127x127	203
HSS152.4x152.4	229
HSS177.8x177.8	254
HSS203.2x203.2	280



FOOTING SCHEDULE

MARK	FOOTING SIZE			CONC. CAP OR PIER			REMARKS
	WIDTH	DEPTH	REINF. B.E.W.	SIZE	VERT. BAR / DWL	TIES	
F1	1600x1600	400	6-15M	500x 500	4-20M	10MΦ 250c/c	
F2	100 x1000	300	3-15M	500x 500	4-15M	10MΦ 250c/c	
F3	900x900	300	3-15M	MASONRY PIER			
F4	1200x1200	300	4-15M	MASONRY PIER			
F5	1600x1000	350	6-15MB SHORT 4-15MB LONG	MASONRY PIER			

PIER SCHEDULE

MARK	SIZE	REINFORCING	REMARKS	CROSSHATCH PIER GRAVEL CONCRETE
P1	400 x 600	4-15MV + DWL GROUTED OR SOLID	MIN. NO. 8 AWG TIES @ 200 c/c MAX.	
P2	190 x 400	2-20MV + DWL	HEAVY DUTY DUROWALL TIES (TRUSS TYPE) @ 200c/c	
P3	240 x 400	2-20MV + DWL	HEAVY DUTY DUROWALL TIES (TRUSS TYPE) @ 200c/c	
P4	400 x 600	4-20MV + DWL	MIN. NO. 8 AWG TIES @ 300c/c MAX.	
P5	290 x 1000	2-15MV + DWLS @ EACH END	HEAVY DUTY DUROWALL TIES (TRUSS TYPE) @ 200c/c	
P6	290 x 900	4-20M + DWLS	MIN. NO. 8 AWG TIES @ 300c/c MAX.	

MARK	DATE	REVISIONS
	FEB. 7 '94	ISSUED FOR TENDER
	AUG. 18/93	ISSUED FOR PRICING

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WOODBRIDGE, ONTARIO, L4L 5Y2
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NOTRE DAME CATHOLIC SCHOOL
FITTONS ORILLIA
SIMCOE COUNTY SEPARATE SCHOOL BOARD

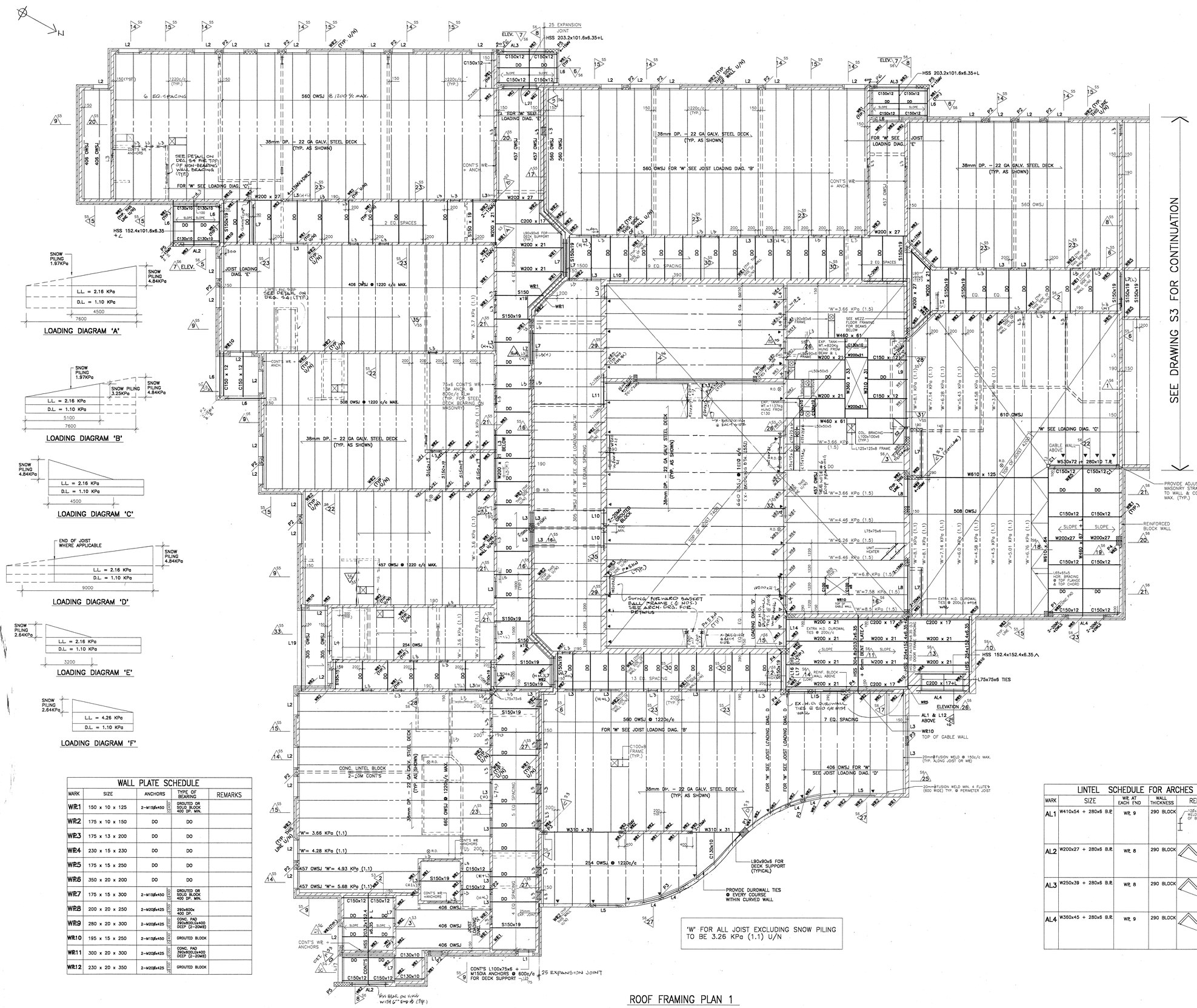
ARCHITECT	R. H. CARTER
DRAWN BY	B. H.
CHECKED BY	W. L.
DATE	JUN. 93
SCALE	1 = 100
PROJECT NO.	93048
ISSUED DATE	

REGISTERED PROFESSIONAL ENGINEER 2/7/94 W. C. K. LEUNG PROVINCE OF ONTARIO

FOUNDATION PLAN 1

DRAWING NO. S1 OF 6

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ROOF FRAMING NOTES :

1. 'W' FOR STEEL JOISTS INCLUDES LIVE LOAD PLUS DEAD LOAD & SELF WEIGHT OF JOISTS.
2. DESIGN LIVE LOAD = 2.16 KPa
3. JOIST DEAD LOADS ARE GIVEN IN BRACKETS ( ).
4. TOP OF STEEL OR JOIST TO BE 3800 ABOVE FINISHED FLOOR UNLESS NOTED OTHERWISE ON PLAN.
5. MINIMUM BRIDGING TO BE 1-L38 x 38 x 3 OR TO SUIT CODE REQUIREMENTS. X-BRIDGING IN C.P. RM.
6. PROVIDE SOLID BLOCK COURSE @ WINDOW/DOOR JAMB (TYPICAL).
7. 'A' DENOTES EXTEND AND CONNECT JOIST BOTTOM CHORD TO BOTTOM FLANGE OF BEAM OR WALL.
8. ~~DESIGN AND CONNECT TIE JOISTS FOR A REVERSIBLE WIND MOMENT OF.~~
9. SEE DRAWING S-4 FOR NOTES AND TYPICAL DETAILS.
10. DESIGN ALL ROOF JOISTS FOR A NET UPLIFT FORCE=0.48KPa WITH 1.52m (LOW ROOF), 1.76m (HIGH ROOF) WIDE AROUND THE PERIMETER OF BUILDING EXCEPT A NET UPLIFT FORCE = 0.24 KPa FOR THE REMAINING AREA OF THE BUILDING.
11. ROOF ASSEMBLY TO CONFORM TO ULC R210 EXCEPT CORRIDORS, GENERAL PURPOSE ROOM & MECHANICAL ROOM.
12. NON-BEARING WALLS ARE NOT LIMITED TO THOSE SHOWN IN DOTTED LINES ON DRAWINGS. PROVIDE LINTELS ACCORDING TO TABLE ON DRAWING S-4.
13. PROVIDE LINTELS (AVERAGE 2 EACH CLASSROOM) 2-L125x90x8 FOR DUCT OPENINGS IN CORRIDOR WALLS (190 WALL).
14. ROOFS ARE TO BE SLOPED FOR DRAINAGE BY RAISING OR VARYING BEARING PLATE LEVELS. SEE ARCHITECTURAL DRAWINGS FOR REQUIRED LEVELS AND SLOPES.
15. ▨ DENOTES FULLY GROUTED BLOCK MIN. 400mm LONG. SEE PLAN FOR REINFORCING STEEL IN CELL.
16. JOISTS ON MECHANICAL ROOM ROOF SHALL BE DESIGNED FOR ONE 1KN VERTICAL LOAD MOVING ALONG TOP/BOT. CHORD OF JOIST.

LINTEL SCHEDULE

MARK	SIZE	WE AT EACH END	WALL THICKNESS	REMARKS
L1	S150x19 + 350x6 B.P. +1-L90x90x6	WE 3	190+90 BLOCK	BEAM CENTRED ON BLOCK WALL
L2	W200x21 + 380x6 B.P. +1-L125x90x8 (LLV)	WE 3	190+90 BLOCK	BEAM CENTRED ON BLOCK WALL
L3	LINTEL BLOCK + 2-15M CONT'S	NIL	190 BLOCK	ALTERNATIVE: 190 LINTEL BLOCK & 2-10M MAX. 1000 CLR. OPG.
L4	2-L125x90x8 (SLV)	NIL	240 BLOCK	
L5	1-L125x125x8 + 1-L125x90x8 (LLV)	NIL	240 BLOCK	
L6	W200x27 + 380x8 B.P. +1-L150x100x8 (LLV)	WE 3	190+90 BLOCK	BEAM CENTRED ON BLOCK WALL
L7	S150x19+ 180x6 B.P.	WE 3	190 BLOCK	DO
L8	W250x33 + 886x8 B.P. +1-L150x100x8 (LLV)	WE 5	190+90 BLOCK	BEAM CENTRED ON BLOCK WALL
L9	W410x54 + 560x8 B.P. +1-L150x100x10 (LLV)	WE 5	190+90 BLOCK	12@300 ROD WELDED TO TOP OF BEAM @ 600 c/c + STIFF B 6mm THK x 100 LONG @ 1200 c/c (L9 ONLY) MIN. 250 BEARING
L10	3-L125x125x8	NIL	390 BLOCK	
L11	2-W150x22 + 380x8 BOT. P.	WE 6	390 BLOCK	16ø BOLT @ 800 c/c
L12	W250x33 + 280x8 B.P.	WE 7	290 BLOCK	ON WALL
L13	W460x61 + 230x6 B.P. +W150 WELDED TO TOP @ 800c/c	WE 4	240 BLOCK	300 LONG
L14	W250x33 + 230x6 B.P.	WE 4	240 BLOCK	MIN. BEARING 250
L15	W410x60 + 380x8 B.P.	WE 11	390 BLOCK	MIN. BEARING 300
L16	W200x31 + 280x8 B.P.	WE 5	290 BLOCK	
L17	W200x27 + 280x6 T.R.	WE 5	290 BLOCK	
L18	W360x45 + 380x10B.P.	WE 7	390 BLOCK	
L19	W310x39 +380x8 B.P. +1-L150x10ux10 (LLV)	WE 7	190+90 BLOCK	SIMILAR TO L9
L20	3-L100x90x6 (LLV)	NIL	290 BLOCK	
L21	W200x27 + 180x6 B.P.	WE 3	190 BLOCK	

WALL PLATE SCHEDULE

MARK	SIZE	ANCHORS	TYPE OF BEARING	REMARKS
WP1	150 x 10 x 125	2-M15@x450	GROUTED OR SOLID BLOCK 400 DP. MIN.	
WP2	175 x 10 x 150	DO	DO	
WP3	175 x 13 x 200	DO	DO	
WP4	230 x 15 x 230	DO	DO	
WP5	175 x 15 x 250	DO	DO	
WP6	350 x 20 x 200	DO	DO	
WP7	175 x 15 x 300	2-M15@x450	GROUTED OR SOLID BLOCK 400 DP. MIN.	
WP8	200 x 20 x 250	2-M20@x425	290x600x 400 DP.	
WP9	280 x 20 x 300	2-M20@x425	CONC. PAD 390x800LDx400 DEEP (2-20M@)	
WP10	195 x 15 x 250	2-M15@x450	GROUTED BLOCK	
WP11	300 x 20 x 300	2-M20@x425	CONC. PAD 390x800LDx400 DEEP (2-20M@)	
WP12	230 x 20 x 350	2-M20@x425	GROUTED BLOCK	

LINTEL SCHEDULE FOR ARCHES

MARK	SIZE	WE AT EACH END	WALL THICKNESS	REMARKS
AL1	W410x54 + 280x6 B.P.	WE 9	290 BLOCK	12ø300 ROD WELDED TO TOP OF BEAM @ 600c/c
AL2	W200x27 + 280x6 B.P.	WE 8	290 BLOCK	20mm R. FOR BEAM & 6mm R FOR L
AL3	W250x39 + 280x6 B.P.	WE 8	290 BLOCK	20mm R. FOR BEAM & 6mm R FOR L
AL4	W360x45 + 280x6 B.P.	WE 9	290 BLOCK	20mm R. FOR BEAM & 6mm R FOR L

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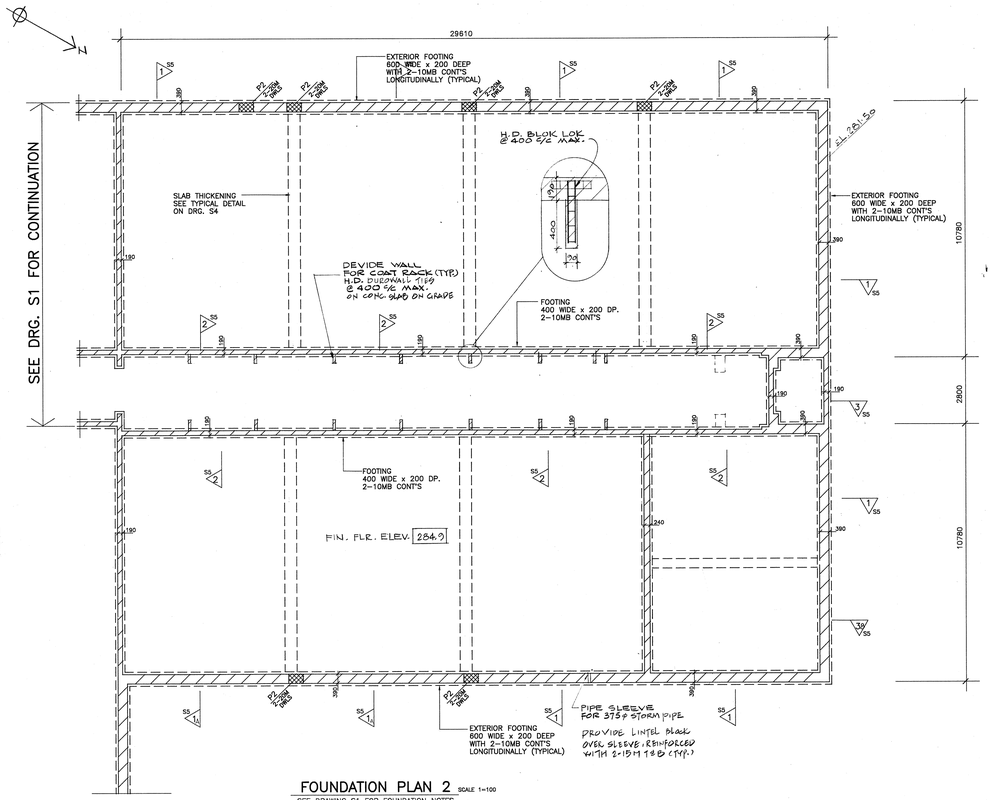
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SIMCOE COUNTY SEPARATE SCHOOL BOARD

ARCHITECT: R. H. CARTER	
DRAWN BY: B. H.	CHECKED BY: W. L.
DATE: JUN. 93	SCALE: 1 = 100
PROJECT NO.: 93048	ISSUED DATE:

REGISTERED PROFESSIONAL ENGINEER W. C. K. LEUNG 4/7/94 PROVINCE OF ONTARIO

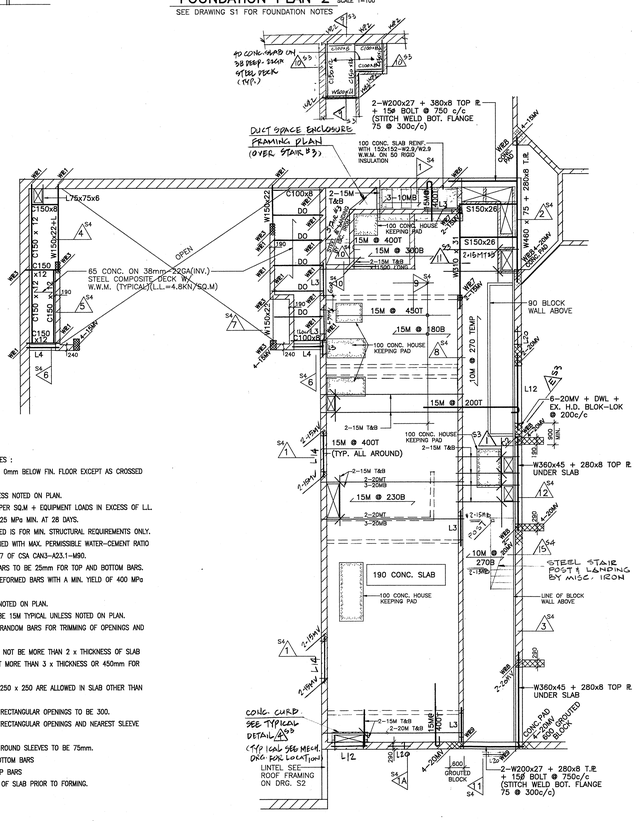
ROOF FRAMING PLAN 1 — DRAWING NO. S2 OF 6

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FOUNDATION PLAN 2 SCALE 1=100

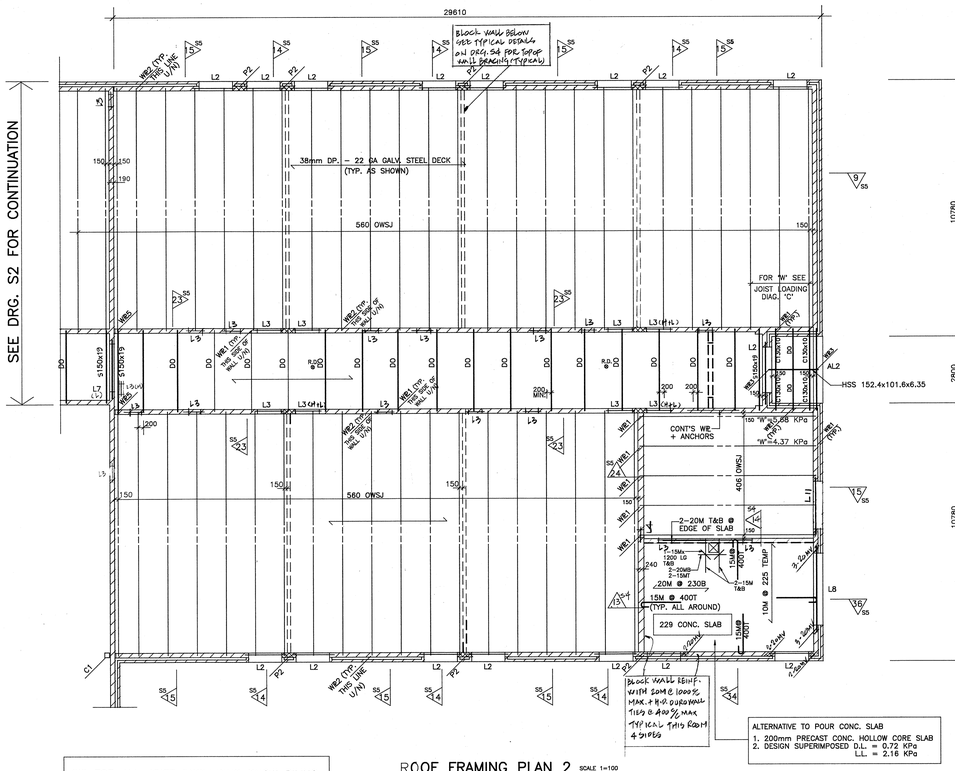
SEE DRAWING S1 FOR FOUNDATION NOTES



MECHANICAL FLOOR FRAMING PLAN SCALE 1=100

MECHANICAL FLOOR FRAMING NOTES :

1. TOP OF STRUCTURAL SLAB IS 0mm BELOW FIN. FLOOR EXCEPT AS CROSSED AND NOTED.
2. SLAB TO BE 190 THICK UNLESS NOTED ON PLAN.
3. DESIGN LIVE LOAD = 4.8KN PER SQ.M + EQUIPMENT LOADS IN EXCESS OF L.L.
4. CONCRETE STRENGTH TO BE 25 MPa MIN. AT 28 DAYS.
5. CONCRETE STRENGTH SPECIFIED IS FOR MIN. STRUCTURAL REQUIREMENTS ONLY. CONCRETE MIX TO BE DESIGNED WITH MAX. PERMISSIBLE WATER-CEMENT RATIO IN ACCORDANCE WITH TABLE 7 OF CSA CAN3-A23.1-M90.
6. CONCRETE COVER FOR RE-BARS TO BE 25mm FOR TOP AND BOTTOM BARS.
7. REINFORCING STEEL TO BE DEFORMED BARS WITH A MIN. YIELD OF 400 MPa GRADE 400.
8. TEMPERATURE STEEL TO BE NOTED ON PLAN.
9. ALL REINFORCING STEEL TO BE 15M TYPICAL UNLESS NOTED ON PLAN.
10. PROVIDE 10M x 1200 LONG RANDOM BARS FOR TRIMMING OF OPENINGS AND PIPE SLEEVES.
11. SPACING OF RE-BARS SHALL NOT BE MORE THAN 2 x THICKNESS OF SLAB FOR TWO WAY SLAB AND NOT MORE THAN 3 x THICKNESS OR 450mm FOR FOR ONE WAY SLAB.
12. NO OPENINGS LARGER THAN 250 x 250 ARE ALLOWED IN SLAB OTHER THAN AS SHOWN ON DRAWING.
13. MINIMUM DISTANCE BETWEEN RECTANGULAR OPENINGS TO BE 300.
14. MINIMUM DISTANCE BETWEEN RECTANGULAR OPENINGS AND NEAREST SLEEVE TO BE 200mm.
15. MINIMUM DISTANCE BETWEEN ROUND SLEEVES TO BE 75mm.
16. LEGEND : B DENOTES BOTTOM BARS
T DENOTES TOP BARS
17. VERIFY ALL EXTERIOR EDGES OF SLAB PRIOR TO FORMING.



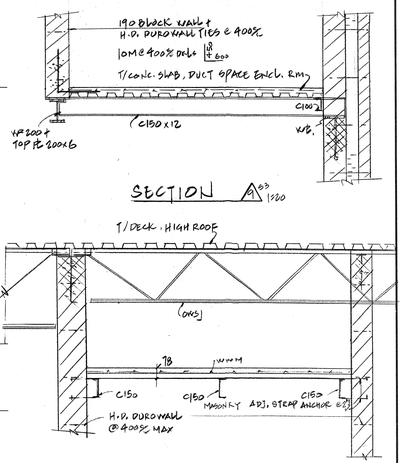
'W' FOR ALL JOIST EXCLUDING SNOW PILING TO BE 3.26 KPa (1.1) U/N

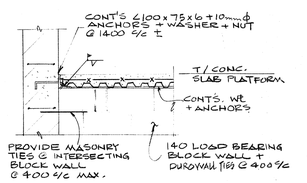
ROOF FRAMING PLAN 2 SCALE 1=100

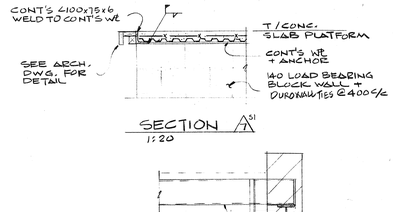
SEE DRAWING S2 FOR ROOF FRAMING NOTES

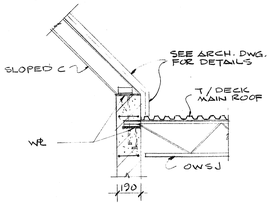
ALTERNATIVE TO POUR CONC. SLAB

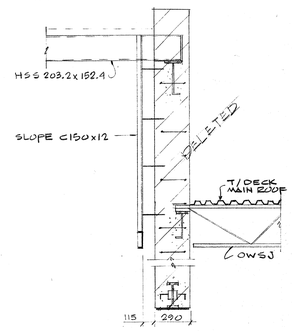
1. 200mm PRECAST CONC. HOLLOW CORE SLAB
2. DESIGN SUPERIMPOSED D.L. = 0.72 KPa
   L.L. = 2.16 KPa

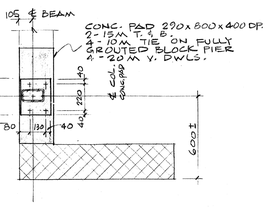


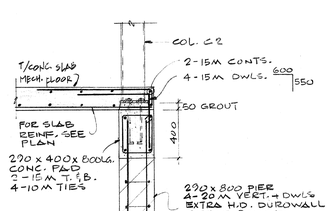


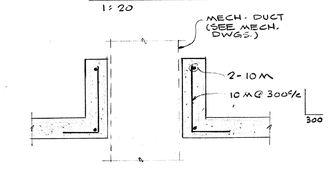












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NOTRE DAME CATHOLIC SCHOOL
FITTONS RD. ORILLIA
SIMCO COUNTY SEPARATE SCHOOL BOARD

ARCHITECT: R. H. CARTER	
DRAWN BY: B. H.	CHECKED BY: W. L.
DATE: JUN. 93	SCALE: 1 = 100
PROJECT NO.: 93048	ISSUED DATE:

REGISTERED PROFESSIONAL ENGINEER W. C. K. LEUNG PROVINCE OF ONTARIO 2/7/94

FOUNDATION PLAN 2, ROOF FRAMING PLAN 2 & MEZZANINE FLOOR FRAMING PLAN

DRAWING NO. S3 OF 8

(A) GENERAL NOTES :

1. THE FOLLOWING NOTES AND THE TYPICAL DETAILS SHOWN ON THE SKETCHES ARE APPLICABLE TO ALL STRUCTURAL CONDITIONS NOT SPECIFICALLY DETAILED ON THE STRUCTURAL DRAWINGS.
2. THE STRUCTURAL DRAWINGS SHALL BE READ WITH THE SPECIFICATIONS AND OTHER CONTRACT DOCUMENTS.
3. ONLY DIMENSIONS RELEVANT TO STRUCTURAL ELEMENTS ARE SHOWN ON THE DRAWINGS.
4. THE DESIGN LIVE LOAD SHOWN UNDER PLANS SHALL NOT BE EXCEEDED DURING CONSTRUCTION. THE CONTRACTOR SHALL MAKE ADEQUATE PROVISIONS FOR CONSTRUCTION LOADS AND FOR ADEQUATE TEMPORARY BRACING TO KEEP THE STRUCTURE (INCLUDING MASONRY WALLS, FLOORS AND ROOF DECKS ETC...) PLUMB AND IN TRUE ALIGNMENT AT ALL PHASES OF CONSTRUCTION.
5. SEE OTHER CONTRACT DOCUMENTS FOR LOCATIONS AND DIMENSIONS OF PITS, BASES, SUMPS TRENCHES, DEPRESSIONS, GROOVES AND CHAMFERS, NOT SHOWN ON STRUCTURAL DRAWINGS
6. ANY INCONSISTANCIES IN DIMENSIONS AND DETAILS BETWEEN STRUCTURAL AND ARCHITECTURAL DRAWINGS ARE TO BE REPORTED TO THE ENGINEER PRIOR TO PROCEEDING WITH WORK.
7. DO NOT SCALE DRAWINGS.
8. NO BACKFILL SHALL BE PLACED AGAINST WALLS UNTIL SLABS BRACING THE TOP AND BOTTOM OF WALLS HAVE BEEN POURED AND SET.
9. ALL SHOP DRAWINGS INCL. STRUCTURAL STEEL, REINFORCING STEEL, PRECAST CONC. PANEL, ETC., TO BE SUBMITTED TO THE ENGINEER FOR REVIEW.

(B) REINFORCING STEEL :

1. ALL REINFORCING TO BE DEFORMED BARS CONFORMING TO C.S.A. G30.12 GRADE 400 UNLESS OTHERWISE NOTED ON PLAN. STIRRUPS AND TIES TO BE DEFORMED BARS TO C.S.A. G30.12 WITH MIN. GRADE 400.
2. ALL REINFORCING SHALL BE ACCURATELY PLACED AND SUPPORTED BY CONTINUOUS METAL OR OTHER APPROVED CHAIRS. IF REQUIRED, ADDITIONAL BARS OR STIRRUPS SHALL BE PROVIDED BY THE CONTRACTOR TO SECURE MAIN BARS AGAINST DISPLACEMENT.
3. REINFORCING IN FOOTINGS, SLABS ON EARTH, AND CONCRETE MEMBERS EXPOSED ARCHITECTURALLY OR TO THE WEATHER SHALL BE SUPPORTED IN THE DESIGNATED POSITION BY SOLID PRE-CAST CONCRETE CHAIRS.
4. MINIMUM CONCRETE COVER FOR REINFORCING :
   (A) FOOTINGS AND OTHER MEMBERS POURED AGAINST EARTH ....75 (3")
   (B) CONCRETE POURED IN FORMS, BUT EXPOSED TO WEATHER OR EARTH :
   BARS LARGER THAN 15M ....50 (2")
   BARS 15M AND SMALLER ....40 (1-1/2")
   (C) CONCRETE NOT EXPOSED TO EARTH OR WEATHER :
   SLABS AND WALLS ....25 (1")
   BEAMS AND GIRDERS ....40 (1-1/2")
   COLUMN MAIN STEEL ....50 (2")
5. SPLICES AT POINTS OF MAXIMUM TENSILE STRESS SHALL BE AVOIDED WHEREVER POSSIBLE; SUCH SPLICES, WHERE USED, SHALL BE APPROVED BY THE ENGINEER AND SHALL BE 36 BAR DIAMETERS.
6. CONTINUOUS AND TEMPERATURE REINFORCING BARS SHALL BE LAPPED 24 BAR DIAMETERS OR 450 (1'-6")MIN. AT SPLICES OR CORNERS. TERMINATE CONTINUOUS BARS AT NON-CONTINUOUS ENDS WITH STANDARD HOOKS.
7. MINIMUM CLEAR DISTANCE BETWEEN PARALLEL BARS SHALL BE GREATER THAN THE FOLLOWING:
   (A) 1-1/2 TIMES BAR DIAMETER.
   (B) 1.33 TIMES MAXIMUM SIZE OF AGGREGATE.
   (C) 25 (1") MINIMUM.

(C) CONCRETE

1. ALL CONSTRUCTION AND DESIGN TO CONFORM TO N.B.C. 1990 AND ALL APPLICABLE C.S.A., A.S.T.M. AND A.C.I. STANDARDS, LATEST EDITIONS. REFER TO PLAN NOTES FOR CONCRETE STRENGTH, FOR STRUCTURAL REQUIREMENTS. REFER TO TABLE 7 OF C.S.A. STANDARD CAN.3-A23.1-M90 FOR MAX. PERMISSIBLE WATER CEMENT RATIOS AND MIN. COMPRESSIVE STRENGTHS FOR DIFFERENT TYPES OF STRUCTURES AND VARYING DEGREES OF EXPOSURE.
2. MINIMUM ONE STRENGTH TEST SHALL BE MADE FOR EACH 75 CU.M. (100 CU. YDS.) OF EACH TYPE OF CONCRETE WITH MINIMUM ONE TEST FOR EACH CONCRETE TYPE ON ANY ONE DAY. EACH STRENGTH TEST SHALL CONSIST OF 4 SPECIMENTS, TWO TO BE TESTED AT 7 DAYS AND 2 AT 28 DAYS. MAXIMUM SLUMP TO BE 75 (3") MAX. FOR SLABS, BEAMS AND WALLS, & 100 (4") MAX. FOR COLUMNS AND FOOTINGS.
3. VERTICAL CONSTRUCTION JOINTS IN BASEMENT WALLS SHALL BE PROVIDED WITH WATER STOPS.
4. VERTICAL CONSTRUCTION JOINTS IN FLOOR SLABS SHALL BE PROVIDED NEAR THE MIDDLE OF THE SPANS OF SLABS OR BEAMS UNLESS OTHERWISE SHOWN ON PLANS OR SECTIONS. CONSTRUCTION JOINTS SHALL BE PLANNED IN ADVANCE AND IN CONSULTATION WITH THE ENGINEER.
5. NO HORIZONTAL CONSTRUCTION JOINTS SHALL BE ALLOWED IN CONCRETE WALLS, BEAMS AND SLABS UNLESS OTHERWISE SHOWN ON SECTIONS OR PLANS.
6. BEAMS, BRACKETS, COLUMN CAPITAL AND HAUNCHES SHALL BE CONSIDERED AS PART OF THE FLOOR SYSTEM AND SHALL BE POURED MONOLITHICALLY WITH THE FLOOR SLAB UNLESS OTHERWISE SHOWN ON THE SECTIONS.
7. CONCRETE CONTRACTOR TO SET ALL LOOSE MEMBERS THAT ARE TO BE EMBEDDED IN THE CONCRETE (e.g. STEEL WALL PLATES, LOOSE LINTELS, PIPE SLEEVES, ETC.).
8. FORMWORK CONTRACTOR TO FORM ALL HOLES, CHASES, ETC. AND TO SET INSERTS, ANCHOR BOLTS AND OTHER EMBEDDED MEMBERS WHICH ARE REQUIRED TO BE HELD IN PLACE BY THE FORMWORK BEFORE POURING CONCRETE. PROVIDE FORMWORK SUFFICIENT TO OBTAIN REQUIRED ARCHITECTURAL FINISHES.
9. PLUMBING SLOTS, HOLES AROUND PIPES, DUCTS OR OTHER ITEMS WHICH PASS INTO OR THROUGH CONCRETE SLAB OR WALL SHALL BE FILLED AND PATCHED TO THE SAME DEPTHS AS THE FLOOR SLAB OR WALL.
10. REFER TO ARCHITECTURAL, MECHANICAL, ELECTRICAL AND PRECAST SHOP DRAWINGS FOR LOCATIONS OF HOLES, CHASES, PLATES, INSERTS AND PIPE SLEEVES. THIS ENGINEER HAS THE FINAL APPROVAL ON SIZES AND LOCATIONS OF HOLES AND PIPE SLEEVES IN SLABS AND WALLS.
11. ALL CONCRETE SURFACES TO BE FINISHED IN ACCORDANCE WITH ARCHITECTURAL REQUIREMENTS.
12. ALL SLAB, BEAM AND JOIST FORMS SHALL BE CAMBERED IN 800 OF SPAN UNLESS OTHERWISE NOTED. THE CAMBER OF THE FORM SHALL BE CHECKED AND ADJUSTED BEFORE THE INITIAL SET OF CONCRETE. SPECIFIED THICKNESS OF CONCRETE MEMBER MUST BE MAINTAINED THROUGHOUT.
13. NO OPENING LARGER THAN 250 x 250 (10" x 10") ARE ALLOWED IN SLAB OTHER THAN SHOWN OF DRAWINGS.
14. MINIMUM DISTANCE BETWEEN RECTANGULAR OPENINGS TO BE 300 (12"). MINIMUM DISTANCE BETWEEN RECTANGULAR OPENINGS AND NEAREST SLEEVE TO BE 200 (8").
15. MINIMUM DISTANCE BETWEEN ROUND SLEEVES TO BE 75 (3").
16. ALL CONCRETE WORK HAS BEEN DESIGNED BY THE ULTIMATE STRENGTH METHOD.
17. SUBMIT CONCRETE MIX DESIGN TO INSPECTION AND TESTING COMPANY FOR REVIEW AND COMMENT PRIOR STARTING CONSTRUCTION.

(D) PRECAST CONCRETE

1. ALL DESIGN, MANUFACTURE AND INSTALLATION TO CONFORM TO N.B.C. 1990 AND C.S.A. CAN 3-A23.4-M78, LATEST EDITION.
2. PRECAST SUPPLIER IS RESPONSIBLE FOR DESIGN OF ALL PRECAST PANELS AND THEIR CONNECTIONS. THESE CONNECTIONS TO INCLUDE THE REQUIRED INSERTS, ANCHORS AND BRACES TO BE EMBEDDED IN, OR CONNECTED TO THE BUILDING STRUCTURE.
3. ALL PRECAST CONCRETE DESIGN AND SHOP DRAWINGS TO BE SUBMITTED FOR REVIEW, CARRYING THE STAMP OF A REGISTERED PROFESSIONAL ENGINEER.
4. ALL CONNECTION TO BE DESIGNED IN SUCH A MANNER AS NOT TO CAUSE LOCAL OVERSTRESSING OF THE SURROUNDING CONCRETE OR STRUCTURAL STEEL AND TO BE WHOLLY SUFFICIENT BY THEMSELVES, WITHOUT ANY FURTHER REINFORCING, UNLESS SO STATED BY THE MANUFACTURER.
5. PROVIDE SLAB SUPPORT AT STEEL COLUMN WHERE REQUIRED.

(E) STEEL DECK :

1. ALL MATERIALS TO CONFORM TO ASTM A-446. GRADE A AND TO CSSBI STANDARD SPECIFICATIONS.
2. ALL DECKS SHALL BE FABRICATED & DESIGNED TO CSA S136 M89 AND TO CSSBI STANDARD SPECIFICATIONS, LATEST EDITION.
3. PROVIDE STEEL DECK WITH A THICKNESS REQUIRED TO CARRY LOADS AS INDICATED ON PLAN.
4. ALL METAL DECK SHALL BE WELDED TO SUPPORTING STEEL WITH 20 (3/4") DIA. FUSION WELDS @ 300 (12") C/C MAX., & 150 (6") C/C MAX. ALONG PERIMETER OF THE STRUCTURE.
5. MAKE ALLOWANCE FOR REINFORCING ALL OPENINGS TO 300 x 300 (12" x 12") AS SHOWN ON ARCHITECTURAL OR MECHANICAL DRAWINGS.

(F) MASONRY :

1. ALL MASONRY MATERIALS AND WORKMANSHIP SHALL CONFORM TO SECTION 4.4 OF THE ONTARIO BUILDING CODE, LATEST EDITION. CSA CAN3-S304.M84 & CSA CAN3-A370.M84
2. ALL LOAD-BEARING CONCRETE BLOCKS TO HAVE AN ULTIMATE COMPRESSIVE STRENGTH OF 10 MPa (1450 P.S.I.)MINIMUM, BASED ON GROSS AREA.
3. MORTAR FOR LOAD-BEARING & EXTERIOR MASONRY WALL TO BE TYPE S OR BETTER WITH MINIMUM AVERAGE COMPRESSIVE STRENGTH OF 12.5 MPa (1800 P.S.I) @ 28 DAYS.
4. MORTAR FOR INTERIOR NON-LOAD-BEARING MASONRY WALL TO BE TYPE N OR BETTER WITH MINIMUM AVERAGE COMPRESSIVE STRENGTH OF 5170 KPa (750 P.S.I) @ 28 DAYS.
5. PROVIDE 3 COURSES OF SOLID BRICK MASONRY OR 1 COURSE OF SOLID OR GROUTED BLOCK, UNDER ALL BEARING PLATES BEARING ON MASONRY FOR A DISTANCE OF NOT LESS THAN 100 (8") PAST BEARING PLATES ON EACH SIDE.
6. ALL INTERSECTING MASONRY WALLS TO HAVE MASONRY BOND OR HEAVY DUTY "BLOK-LOK" OR EQUIVALENT @ 400 (16") O.C.MAX. VERTICALLY
7. FOR BONDING OF BRICK AND BLOCK USE HEAVY-DUTY TRUSS-TYPE "BLOK-LOK" OR EQUIVALENT @ 400 (16") O.C.MAX. VERTICALLY, COMPLETELY EMBEDDED IN MORTAR.
8. PROVIDE LINTELS OVER ALL OPENINGS AND RECESSES.
9. MINIMUM BEARING ON MASONRY WALLS TO BE AS FOLLOWS :
   STEEL BEAM ....200 (8")
   CONCRETE BEAM ....200 (8")
   STEEL LINTELS....................150 (6")
   CONC. OR REINF. BLOCK LINTELS....................200 (8")
   CONCRETE SLAB....................100 (4")
   CONT'S CONCRETE SLAB....................200 (8")
10. PROVIDE MIN. 25 (1") GROUT UNDER ALL WALL PLATES.
11. FOR CORROSION PROTECTION : USE HOT DIP GALVANIZED WIRE REINFORCEMENT IN EXTERIOR MASONRY WALL AND MILL GALVANIZED WIRE REINFORCEMENT IN INTERIOR WALL.

(G) STRUCTURAL STEEL :

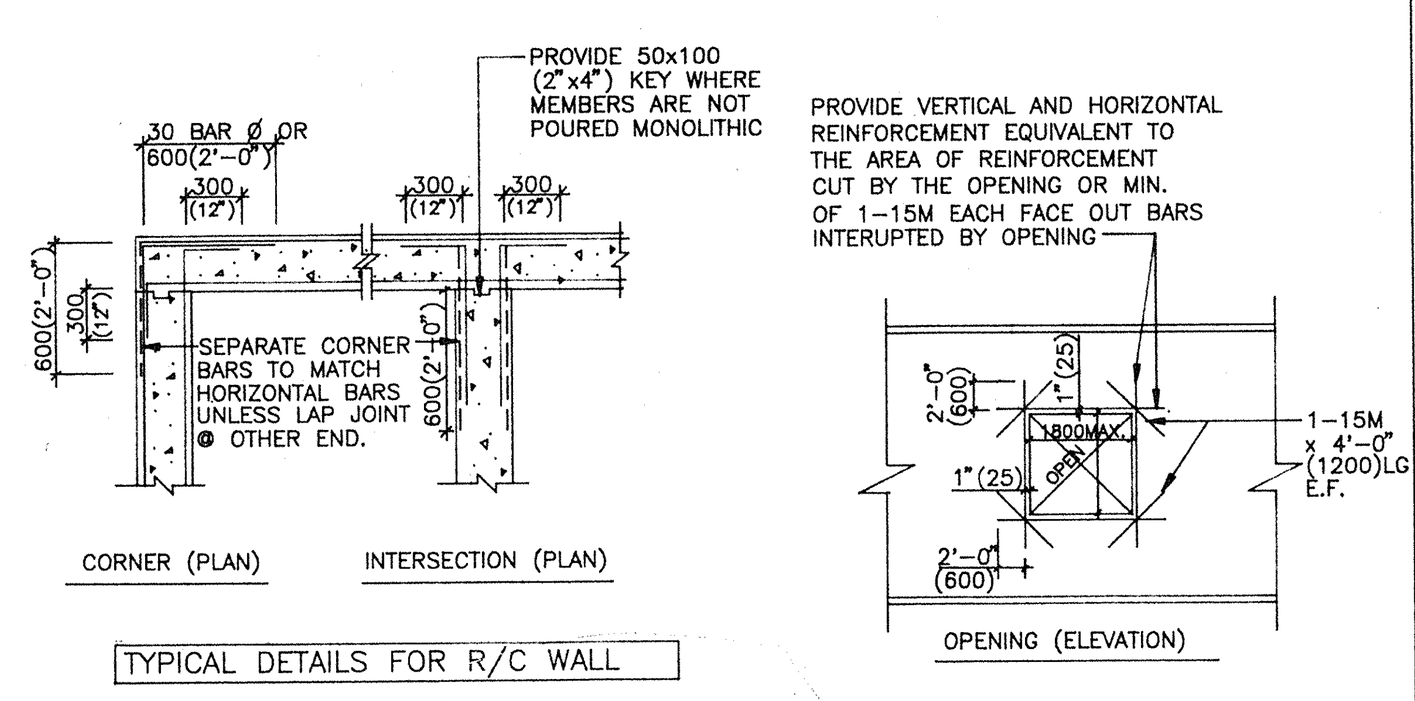
1. ALL STRUCTURAL STEEL TO BE NEW MATERIAL, C.S.A G.40.21 GRADE 300W WITH A MINIMUM YIELD STRENGTH OF 44 K.S.I. UNLESS OTHER WISE NOTED. ALL H.S.S. TO BE C.S.A. G40.20 GRADE 350W CLASS H.
2. FABRICATION AND ERECTION TO CONFORM TO C.S.A. CAN3-S16.1-M89.
3. ALL STRUCTURAL STEEL SHOP DRAWINGS TO BE SUBMITTED TO THE ENGINEER FOR REVIEW PRIOR TO FABRICATION. (SHOP DETAILS INCLUDED).
4. CALCULATIONS FOR CONNECTIONS AND SHOP DETAILS, BEARING THE STAMP OF A REGISTERED PROFESSIONAL ENGINEER, TO BE SUBMITTED TO THE ENGINEER PRIOR TO FABRICATION AS REQUESTED.
5. NO HOLES TO BE PUT IN BEAM FLANGES IN TENSION.
6. WHERE REQUIRED. PROVIDE HOLES IN MEMBERS FOR ATTACHMENT OF OTHER MATERIALS.
7. NO SPLICES IN BEAMS AND COLUMNS ALLOWED UNLESS APPROVED BY THE ENGINEER. BUTT WELES IN SUCH SPLICES & MOMENT CONNECTIONS TO BE X-RATED OR ULTRASONICLY TESTED BY AN INDEPENDENT TESTING COMPANY.
8. WELD COLUMN ALL AROUND TO CAP AND BASE PLATES.
9. FOR BEAMS CANTILEVERED OVER COLUMNS:
   (A) USE 10 (3/8") STIFFENERS IN ROOF BEAMS OVER WEB, OR ONE COLUMN FLANGE.
   (B) USE 10 (3/8") STIFFENERS IN FLOOR BEAMS OVER WEB OR BOTH COLUMN FLANGE.
   (C) USE 25 (1") CAP PLATE & 4 - M20 (3/4") DIA. HIGH TENSILE BOLTS FOR ALL COLUMNS.
10. PROVIDE 38 x 6 x 200 LONG STRAP ANCHORS & 2000 (6'-8") C/C MAX. FOR ALL BEAMS & JOISTS IN CONTACT WITH OR ADJACENT TO MASONRY UNLESS OTHERWISE NOTED.
11. PROVIDE ADJUSTABLE ANCHORS & 450 (1'-6") C/C MAX. VERTICALLY FOR ALL COLUMNS IN CONTACT WITH OR ADJACENT TO MASONRY UNLESS OTHERWISE NOTED.
12. PROVIDE DECK SUPPORT AT COLUMNS WHERE REQUIRED.
13. CANTILEVER BEAMS ARE BY PLASTIC DESIGN.

(H) OPEN-WEB STEEL JOISTS :

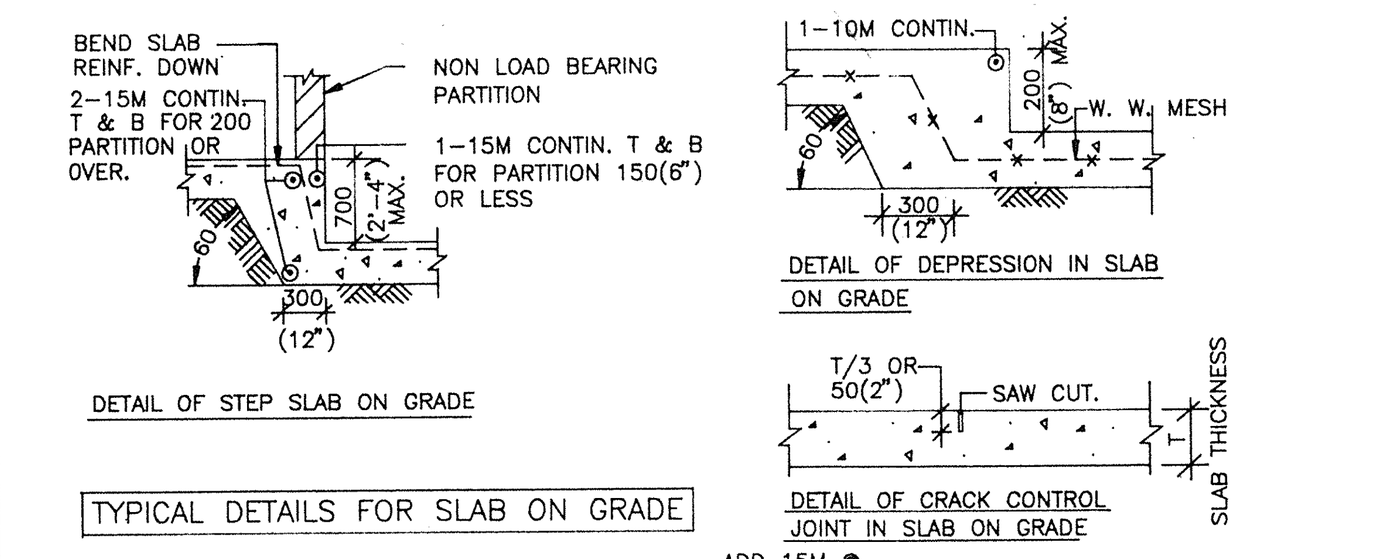
1. ALL JOISTS TO BE DESIGNED & FABRICATED IN ACCORDANCE WITH THE ONTARIO BUILDING CODE & ALL APPLICABLE C.S.A SPECIFICATIONS. LATEST EDITION.
2. ALL JOISTS TO BE CAPABLE OF SUPPORTING THE LOADS NOTED ON PLAN & OF CARRYING UPLIFT FORCES OR PARTIAL LIVE LOADS AS SPECIFIED IN THE ONTARIO BUILDING CODE.
3. DESIGN CALCULATIONS & JOIST DETAILS INCLUDING WELDING & CAMBERS ETC., BEARING THE STAMP OF A REGISTERED PROFESSIONAL ENGINEER, TO BE SUBMITTED TO THE ENGINEER PRIOR TO FABRICATION.
4. CAMBER FOR JOISTS = DEFLECTION OF DEAD LOAD + POINT LOADS + 1/2 LIVE LOADS.
5. MAXIMUM LIVE LOAD DEFLECTION FOR ROOF SEE NOTE # 17 & MAXIMUM TOTAL LOAD DEFLECTION FOR FLOOR TO BE SPAN/360.
6. ALL JOISTS TO BE WELDED TO SUPPORT BEAMS & COLUMNS.
7. JOIST SHOES TO BE DETERMINED BY JOIST SUPPLIER.
8. CONNECT TOP AND BOTTOM CHORD OF JOISTS ON COLUMN CENTRE LINE TO COLUMNS.
9. BEARING FOR JOIST SHOES :
   65 (2-1/2") ON STEEL BEAMS
   100 (4") ON CONCRETE OR MASONRY WALLS.
10. JOIST BRIDGIMG TO CONFORM TO C.S.A. S16.1-M89 WELDED TO EACH JOIST AND ANCHORS TO BEAM OR WALL AT END OF RUN.
11. JOISTS MADE OF COLD-FORMED OF STEEL TO HAVE CHORD MEMBERS NOT LESS THAN 5 (3/16") THICK.
12. JOIST SUPPLIER TO WELD PLATES OR ANGLES TO JOIST TO RECEIVE BRACE, HANGERS ETC.
13. PROVIDE CEILING EXTENSION FOR JOISTS WHERE REQUIRED BY ARCHITECT.
14. ALL POINT LOADS INDICATED ON PLAN TO BE DESIGNED FOR ANYWHERE ALONG THE SPAN OF THE JOIST WITHOUT ADDING DIAGONALS.
15. ALL MECHANICAL UNITS & SPRINKLER LINES 125 (5")DIA. OR LARGER, TO BE REPORTED TO THE ENGINEER FOR INCORPORATION THIER LOADS ON PLAN.
16. ALL PIPES TO BE SUPPORTED ON TOP CHORD AT MID-SPAN OR 1/3 OF SPAN NEAR BRIDGING LINES AND AT PANEL POINTS OF JOIST.
17. LIVE LOAD DEFLECTION FOR ROOF :
   L/240 FOR WAREHOUSE & PLANT
   L/300 FOR OFFICE (SPAN < 30'-0")
   L/360 FOR OFFICE (SPAN > 30'-0")

(I) WIND BEARING METAL STUDS :

1. ALL METAL STUD TO BE NEW MATERIAL, DESIGN, FABRICATION AND INSTALLATION TO CONFORM TO CSA CAN3-S136-M84, LATEST EDITION.
2. MINIMUM THICKNESS OF STEEL STUD AND TRACK TO BE 22mm (18 GAUGE) x 100 DP. @ 400 c/c AND SHALL BE DESIGNED FOR MINIMUM WIND LOAD OF 1.27 KPa.
3. THE SPACING OF TRACK ANCHORS TO BE MAXIMUM @ 800 c/c. THE TRACK THICKNESS SHOULD NOT BE LESS THAN THE THICKNESS OF STEEL STUD.
4. MAXIMUM DEFLECTION OF STUD FOR WIND LOAD TO BE SPAN/300.
5. THE MAXIMUM SPACING OF STEEL BRIDGING SHOULD BE 1220mm. CLIP ANGLES USED IN STEEL BRIDGING CONNECTION TO BE 18 GAUGE OR THICKER.
6. MINIMUM ONE SELF-DRILLING SHEET METAL SCREW IN EACH PLANGE SHALL BE PROVIDED FOR THE END OF STUD TO SUPPORT TRACK CONNECTION.
7. MINIMUM OF FOUR SCREWS FOR SCREWED BRIDGING TO STUD CONNECTIONS.
8. SUBMIT SHOP DRAWINGS SHOWING ALL FRAMING AND TYPICAL CONNECTION DETAILS. AFFIX SEAL OF STRUCTURAL ENGINEER LICENCED TO PRACTICE IN THE PROVINCE OF ONTARIO TO SHOP DRAWING AND DESIGN DATA.

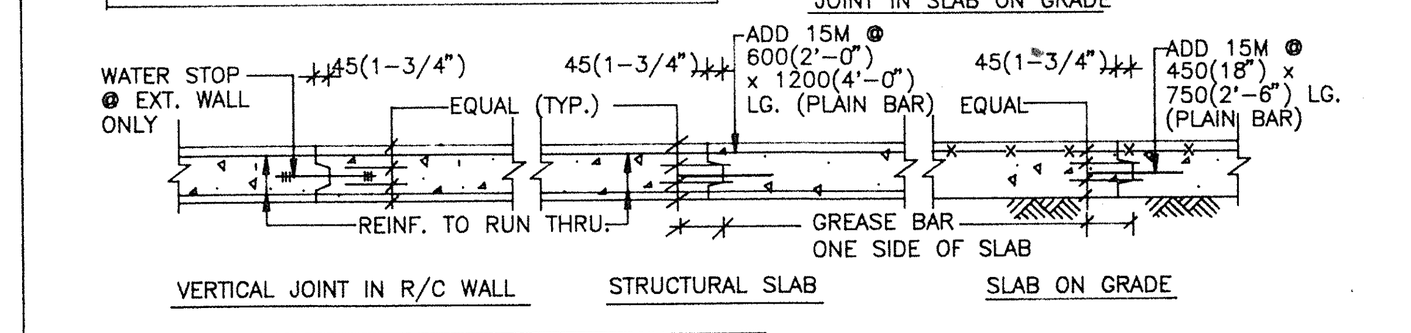


TYPICAL DETAILS FOR R/C WALL

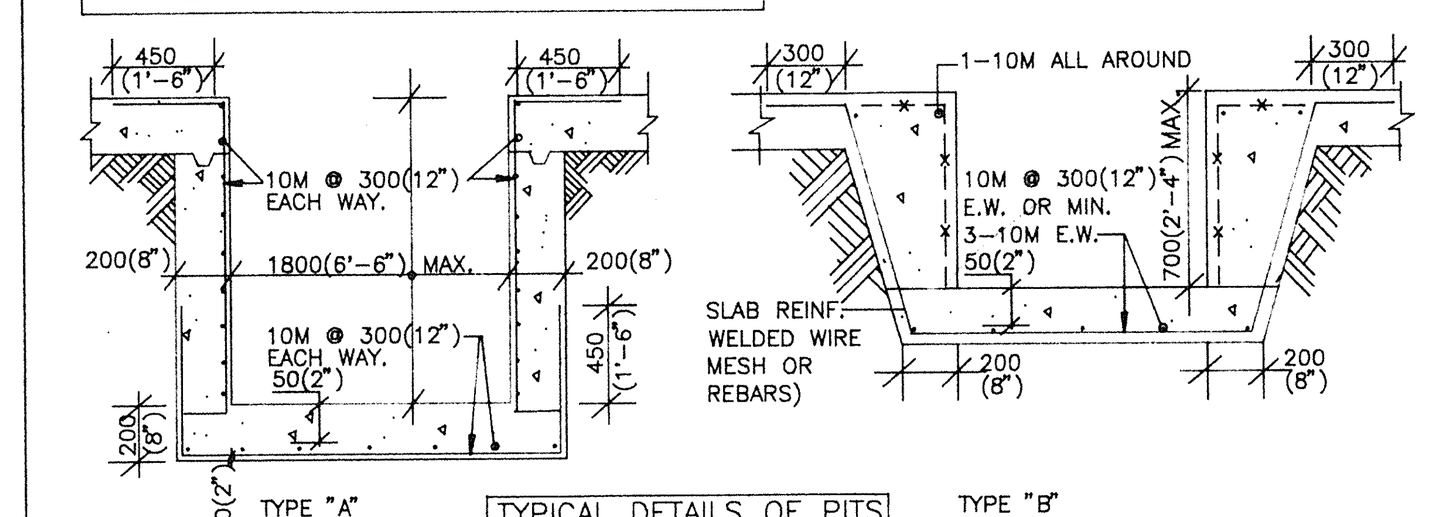


DETAIL OF STEP SLAB ON GRADE / DETAIL OF DEPRESSION IN SLAB ON GRADE / DETAIL OF CRACK CONTROL JOINT IN SLAB ON GRADE

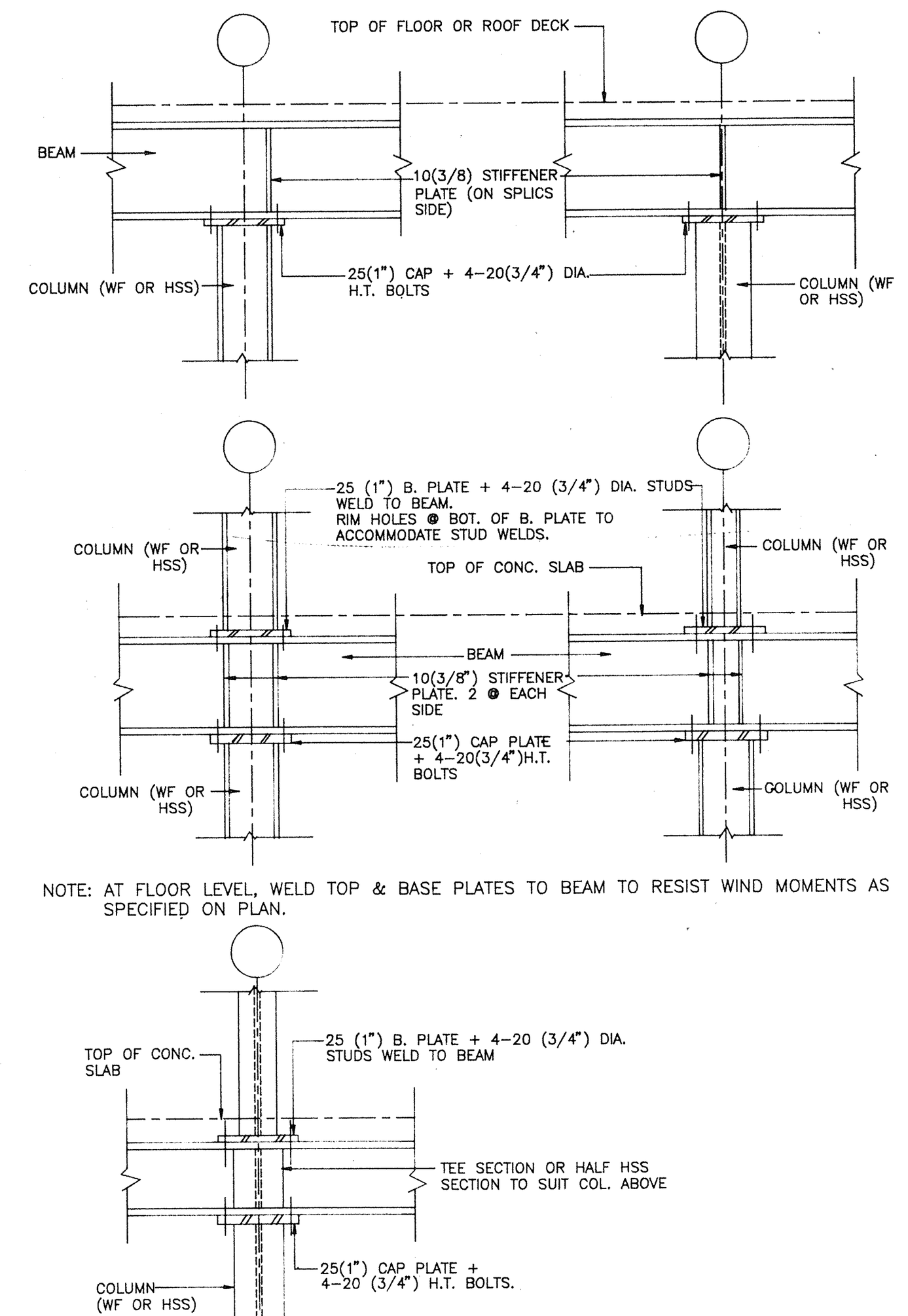
TYPICAL DETAILS FOR SLAB ON GRADE



TYPICAL DETAILS OF CONSTRUCTION JOINTS

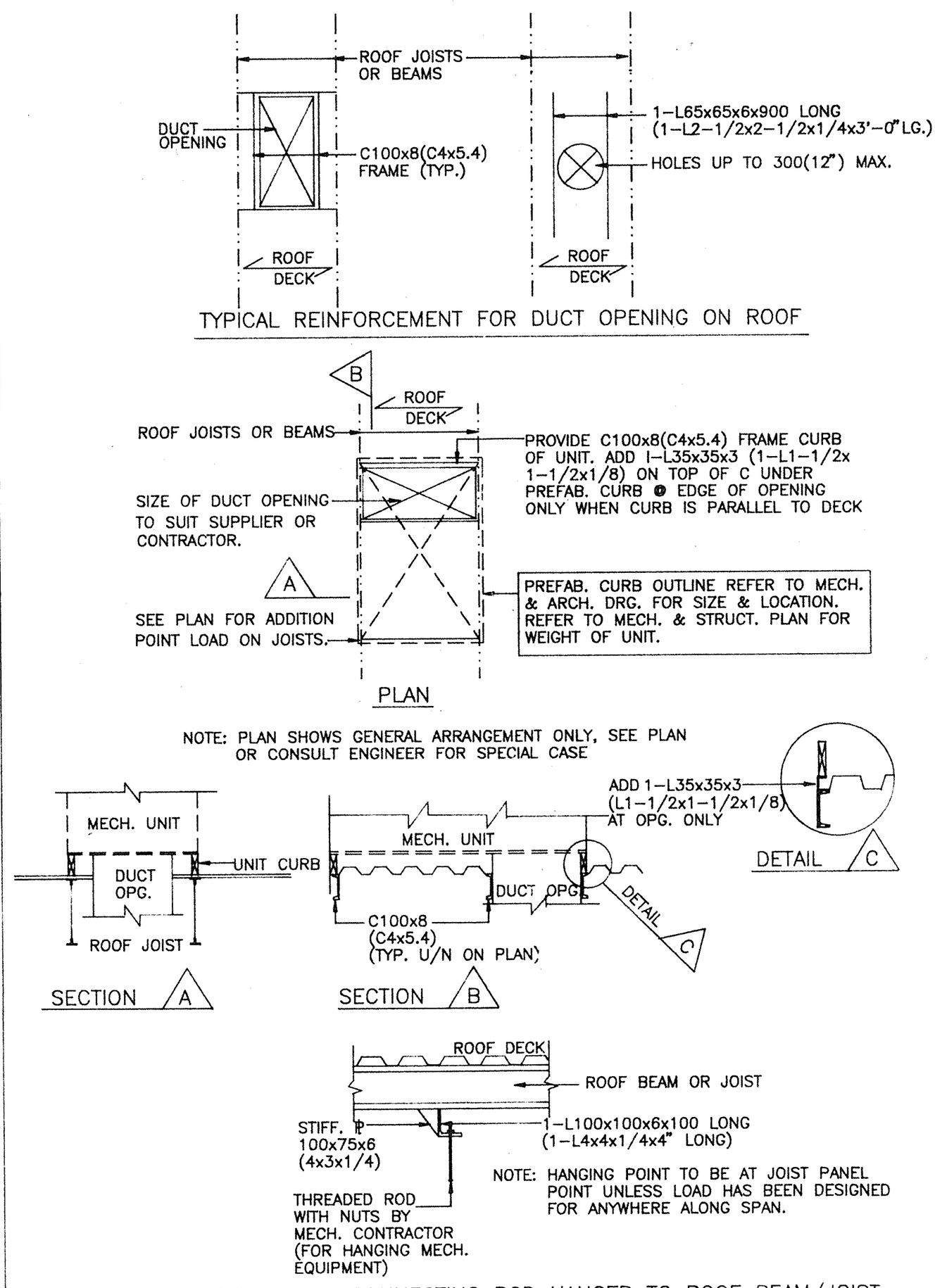


TYPICAL DETAILS OF PITS



NOTE: AT FLOOR LEVEL, WELD TOP & BASE PLATES TO BEAM TO RESIST WIND MOMENTS AS SPECIFIED ON PLAN.

TYPICAL DETAILS OF BEAM-COLUMN JOINT

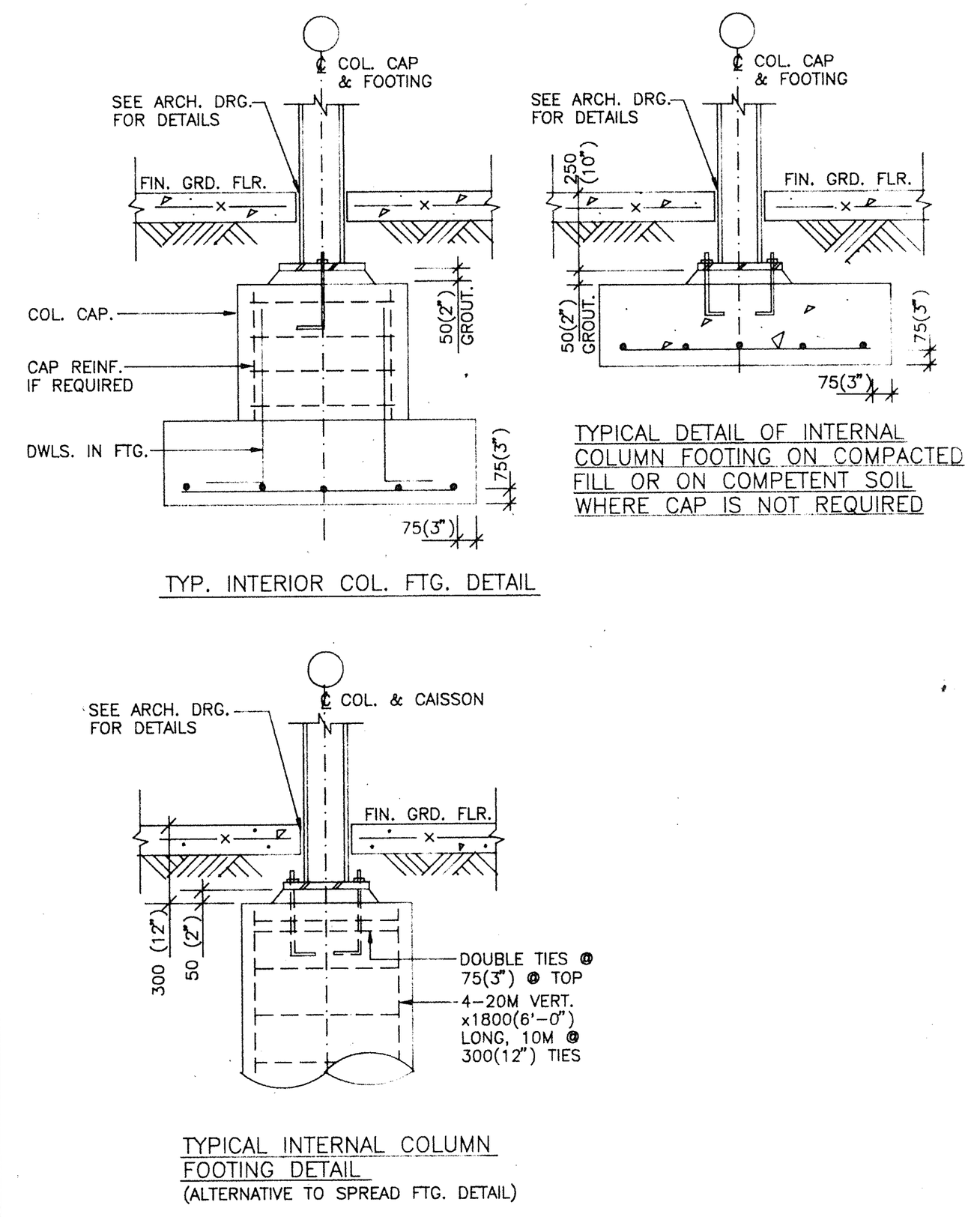


TYPICAL REINFORCEMENT FOR DUCT OPENING ON ROOF

NOTE: PLAN SHOWS GENERAL ARRANGEMENT ONLY, SEE PLAN OR CONSULT ENGINEER FOR SPECIAL CASE

TYPICAL DETAILS OF CONNECTING ROD HANGER TO ROOF BEAM/JOIST

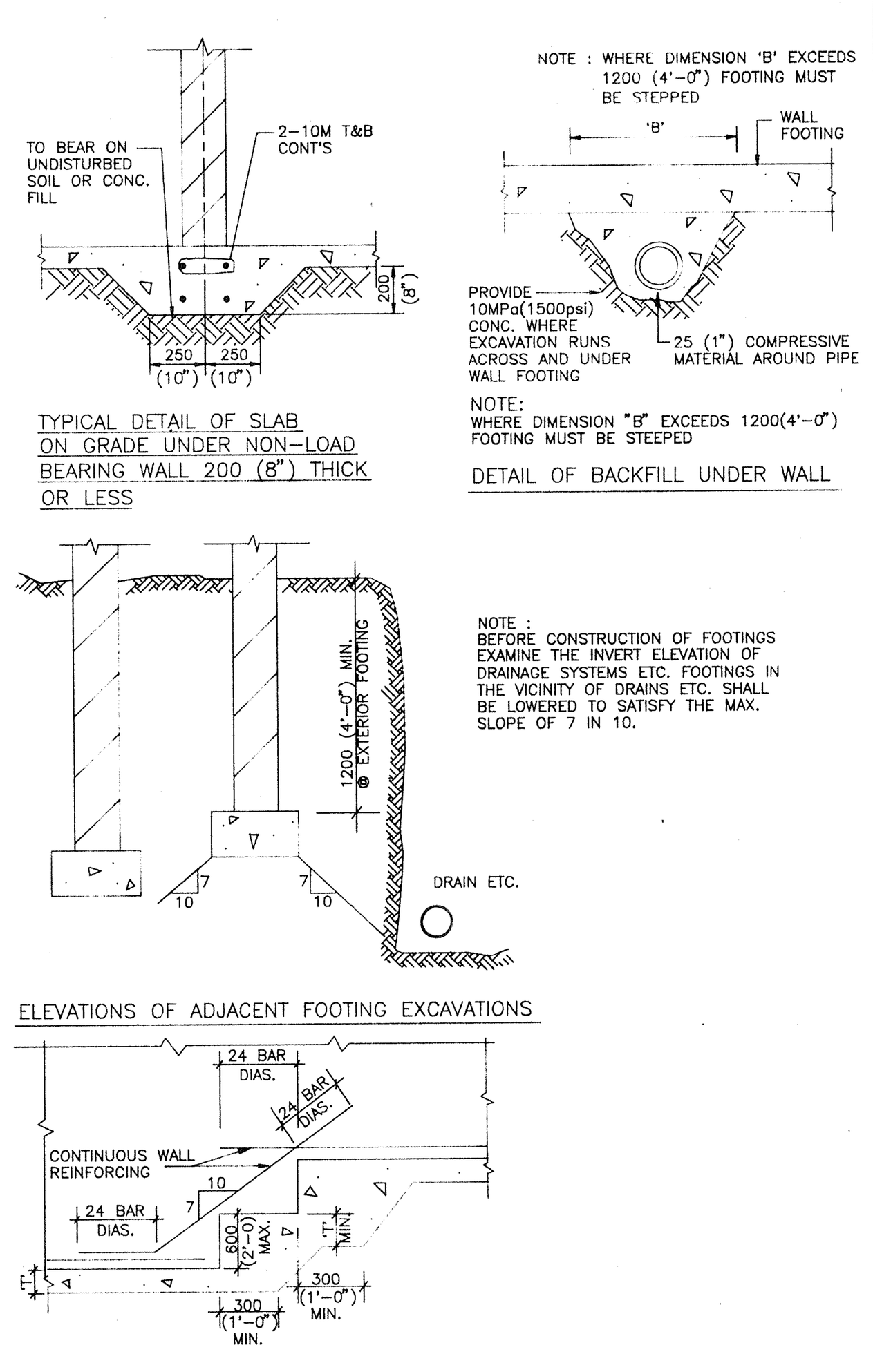
TYPICAL DETAILS OF SUPPORTS FOR A/C UNITS WITH PREFAB. CURBS



TYP. INTERIOR COL. FTG. DETAIL

TYPICAL DETAIL OF INTERNAL COLUMN FOOTING ON COMPACTED FILL OR ON COMPETENT SOIL WHERE CAP IS NOT REQUIRED

TYPICAL INTERNAL COLUMN FOOTING DETAIL (ALTERNATIVE TO SPREAD FTG. DETAIL)



TYPICAL DETAIL OF SLAB ON GRADE UNDER NON-LOAD BEARING WALL 200 (8") THICK OR LESS

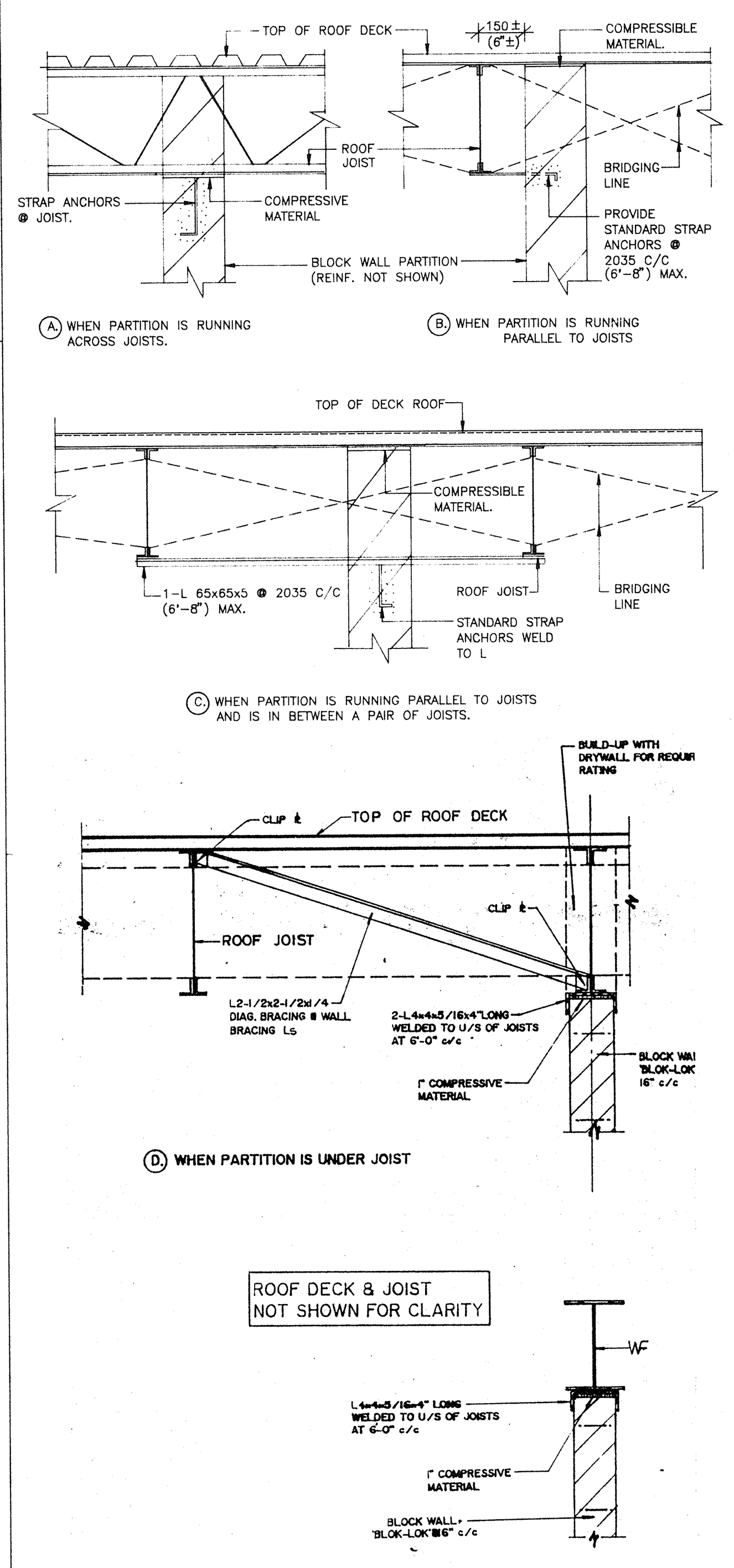
NOTE: WHERE DIMENSION "B" EXCEEDS 1200(4'-0") FOOTING MUST BE STEEPED

DETAIL OF BACKFILL UNDER WALL

NOTE : BEFORE CONSTRUCTION OF FOOTINGS EXAMINE THE INVERT ELEVATION OF DRAINAGE SYSTEMS ETC. FOOTINGS IN THE VICINITY OF DRAINS ETC. SHALL BE LOWERED TO SATISFY THE MAX. SLOPE OF 7 IN 10.

ELEVATIONS OF ADJACENT FOOTING EXCAVATIONS

TYPICAL DETAIL OF STEPPED FOOTING



(A) WHEN PARTITION IS RUNNING ACROSS JOISTS.

(B) WHEN PARTITION IS RUNNING PARALLEL TO JOISTS

(C) WHEN PARTITION IS RUNNING PARALLEL TO JOISTS AND IS IN BETWEEN A PAIR OF JOISTS.

(D) WHEN PARTITION IS UNDER JOIST

ROOF DECK & JOIST NOT SHOWN FOR CLARITY

(E) WHEN PARTITION IS UNDER BEAM

TYPICAL DETAILS FOR BRACING NON-LOAD BEARING WALL

STEEL LINTELS FOR NON-LOAD-BEARING WALL

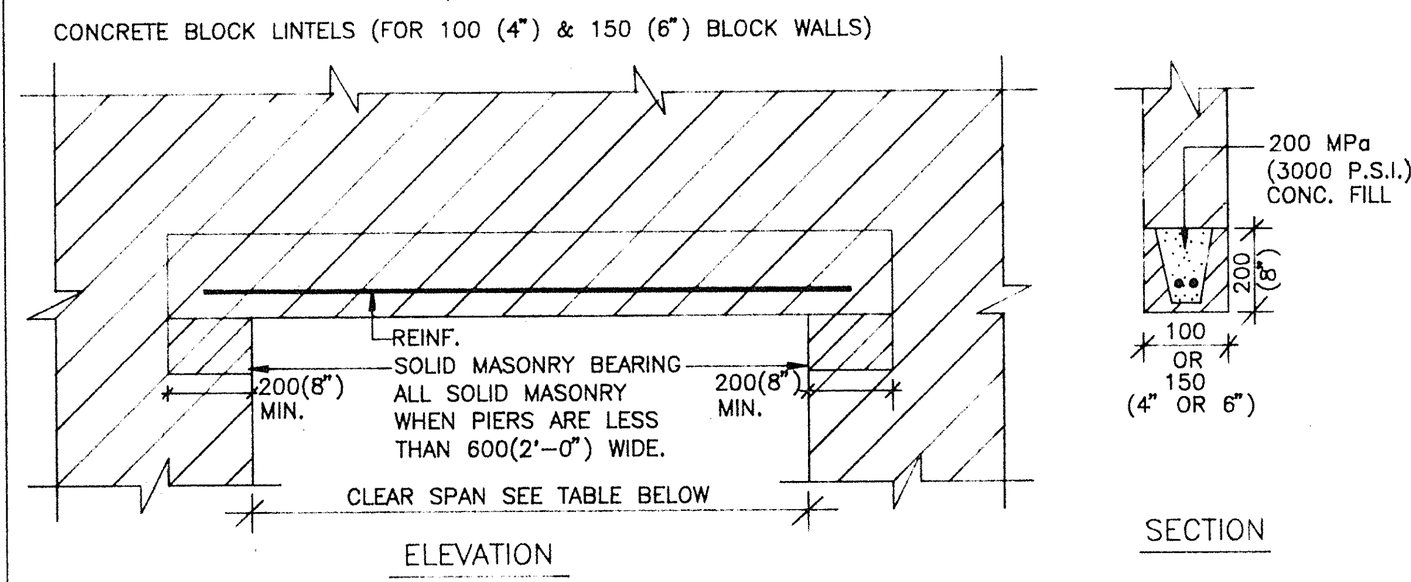
CLEAR SPAN	200 (8") WALL TYPE	200 (8") WALL MATERIAL	250 (10") WALL TYPE	250 (10") WALL MATERIAL	300 (12") WALL TYPE	300 (12") WALL MATERIAL
UP TO 1200 (4'-0")	JL	90x90x6 (3-1/2x3-1/2x1/4)	JL	90x90x6+ 125x90x8 SLV (3-1/2x3-1/2x1/4+ 5x3-1/2x5/16)	JLL	90x90x6 (3-1/2x3-1/2x1/4)
1200 TO 1500 (5'-0")	JL	100x90x6 (4x3-1/2x1/4)	JL	100x90x6+ 125x90x8 SLV (4X3-1/2X1/4+ 5x3-1/2x5/16)	JLL	100x90x6 (4x3-1/2x1/4)
1500 TO 2100 (7'-0")	JL	125x90x8 (5x3-1/2x5/16)	JL	125x90x8+ 125x125x8 (5x3-1/2x5/16+ 5x5x5/16)	JLL	125x90x8 (5x3-1/2x5/16)

90 AND 125 LEGS ARE HORIZONTAL

NOTES :

(1) DOUBLE ANGLES SHALL BE STICHWELDED BACK TO BACK 1-5 (3/16") WELD 25 (1") LONG PER 600 (2'-0") LENGTH TOP AND BOTTOM.
(2) WHERE ANGLES DO NOT BEAR AT THE SAME ELEVATION USE STEEL PACKING TO ENSURE EVEN BEARING.
(3) FOR WALLS THICKER THAN 300 (12") ADD 1 ANGLE OF SIZE SPECIFIED FOR 300 (12") WALL PER ADDITIONAL 100 (4") THICKNESS OF WALL.

CONCRETE BLOCK LINTELS (FOR 100 (4") & 150 (6") BLOCK WALLS)



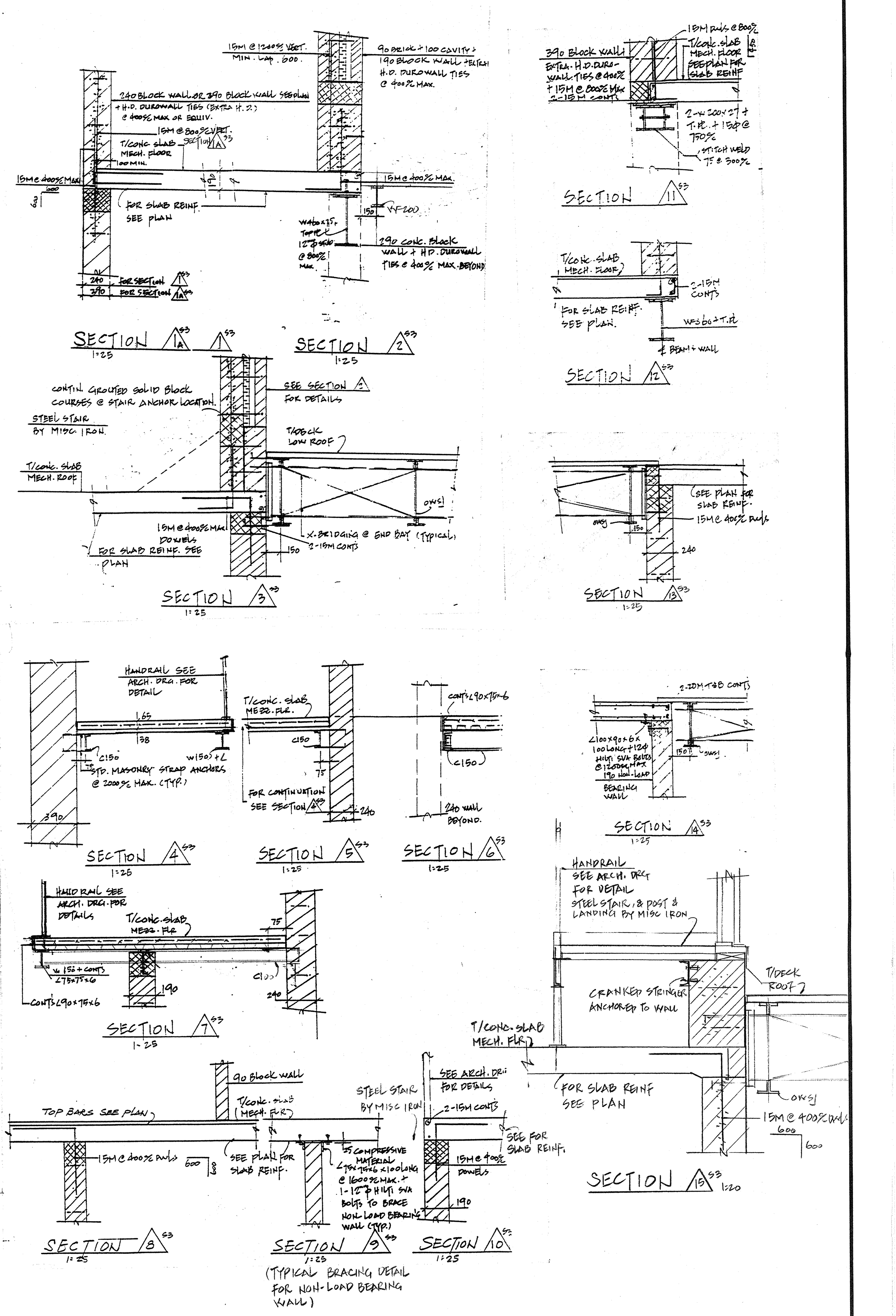
ELEVATION

SECTION

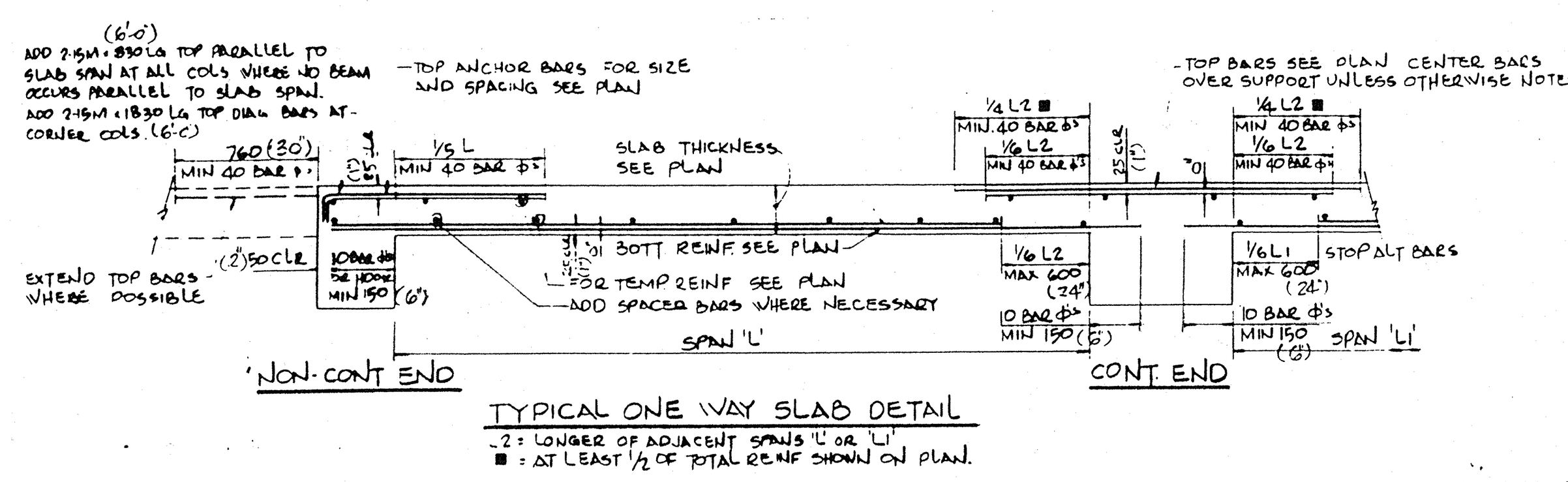
	CLEAR SPAN	REINFORCEMENT
100 (4") WALLS	UP TO 1800 (6'-0")	1 - 10M
150 (6") WALLS	UP TO 1800 (6'-0") 1800 (6'-0") TO 2100 (7'-0")	2 - 10M 2 - 15M
200 (8") WALLS	UP TO 1800 (6'-0") 1800 (6'-0") TO 2100 (7'-0")	2 - 15M 2 - 20M

NOTES FOR REINFORCED MASONRY WALLS

1. ALL CONSTRUCTION TO CONFORM TO OBC 1985 AND LATEST EDITIONS OF ALL APPLICABLE ASTM & C.S.A. SPECIFICATIONS.
2. CONCRETE BLOCKS TO BE NORMAL HOLLOW BLOCKS HAVING AN ULTIMATE COMPRESSIVE STRENGTH OF 1450 P.S.I. (10 mPa) MIN. (f'm).
3. MORTAR TO BE TYPE "S" OR BETTER WITH MINIMUM AVERAGE COMPRESSIVE STRENGTH OF 1800 P.S.I. @ 28 DAYS, (12.5 mPa), COMPOSED BY VOLUME IN PROPORTIONS AS BELOW:
   1/2 PART PORTLAND CEMENT
   1/2 PART MASONRY CEMENT
   2 1/4 TO 3 PARTS SAND
4. GROUT TO HAVE COMPRESSIVE STRENGTH (f'c) OF 2500 P.S.I. (17 mPa) @ 28 DAYS.
5. ALL CELLS CONTAINING REINFORCING STEEL SHALL BE FILLED WITH GROUT IN LIFTS NOT EXCEEDING 4 FT. (1200). GROUT SHALL BE CONSOLIDATED BY TAMPING OR VIBRATING DURING POURING.
6. HORIZONTAL CONSTRUCTION JOINTS SHALL BE FORMED BY STOPPING THE GROUT 1 1/2" (38) BELOW THE TOP BLOCK.
7. ALL REINFORCING STEEL TO CSA G30.12, GRADE 400.
8. SIX MORTAR CUBE SPECIMENS, THREE FROM EACH OF TWO DIFFERENT LOCATIONS, SHALL BE MADE FOR EACH 5,000 SQUARE FEET (4600 m²) OF WALL, CURED AND TESTED, TWO @ 7 DAYS AND FOUR @ 28 DAYS.
9. MORTAR TEST CUBES SHALL BE MADE, CURED AND TESTED IN ACCORDANCE WITH CSA A179 LATEST EDITION "MORTAR FOR UNIT MASONRY".
10. GROUT TO BE TESTED IN ACCORDANCE WITH NOTE 8 AND 9.
11. INSPECTION BY THIS OFFICE IS MANDATORY. NOTIFY THIS OFFICE AT LEAST 24 HOURS BEFORE ERECTION OF WALL.



SECTION 1A, SECTION 1, SECTION 2, SECTION 3, SECTION 4, SECTION 5, SECTION 6, SECTION 7, SECTION 8, SECTION 9 (TYPICAL BRACING DETAIL FOR NON-LOAD BEARING WALL), SECTION 10, SECTION 11, SECTION 12, SECTION 13, SECTION 14, SECTION 15



TYPICAL ONE WAY SLAB DETAIL

L2 : LONGER OF ADJACENT SPANS 'L' OR 'L1'
■ : AT LEAST 1/2 OF TOTAL REINF SHOWN ON PLAN.

MARK	DATE	REVISIONS
	FEB. 7 '94	ISSUED FOR TENDER
	AUG 18 '93	ISSUED FOR PRICING

WILLIAM LEUNG & ASSOCIATES ENGINEERS LTD.
190 MARYCROFT AVENUE, UNIT 12, WOODBRIDGE, ONTARIO, L4L 5Y2
TEL. (416) 851-9535

NOTRE DAME CATHOLIC SCHOOL
FITTONS RD. ORILLIA
SIMCOE COUNTY SEPARATE SCHOOL BOARD

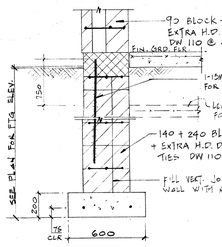
ARCHITECT: R. H. CARTER
DRAWN BY: B.H. CHECKED BY: W. L.
DATE: JUNE 1993 SCALE:
PROJECT NO. 93048 ISSUED DATE:

REGISTERED PROFESSIONAL ENGINEER W. C. K. LEUNG PROVINCE OF ONTARIO 2/7/94

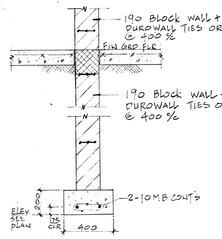
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GENERAL NOTES AND TYPICAL DETAILS

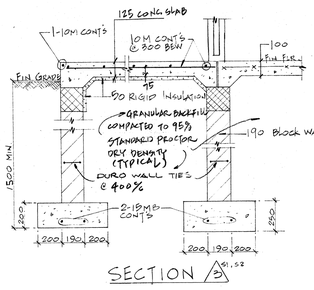
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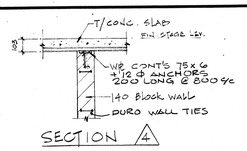
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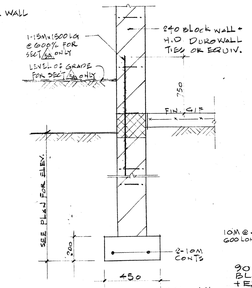
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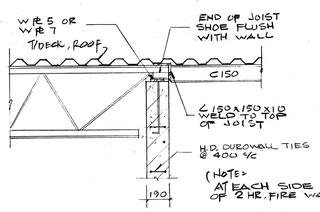
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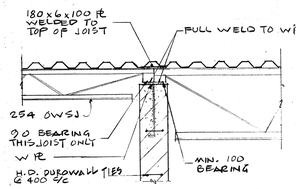
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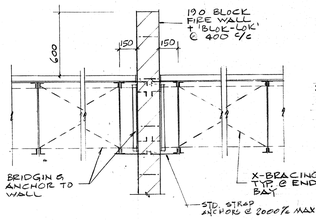
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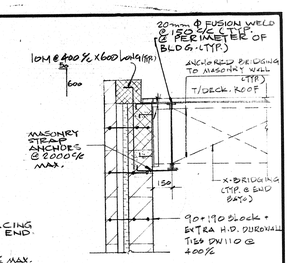
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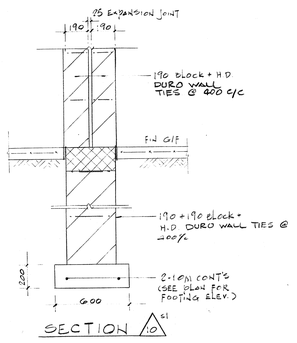
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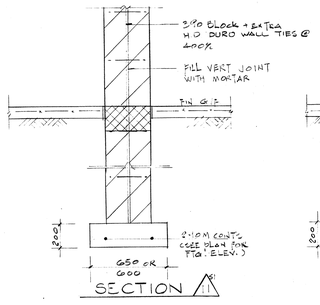
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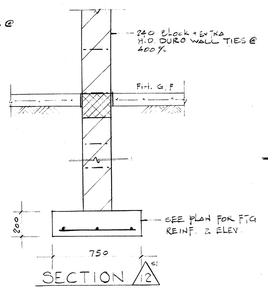
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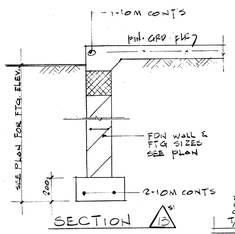
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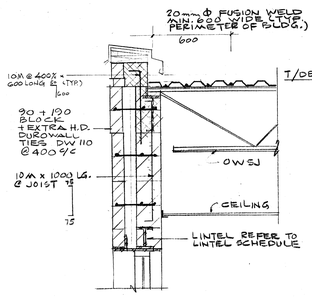
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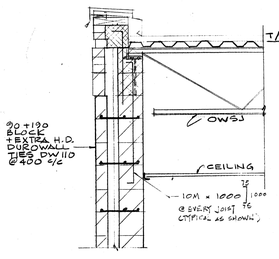
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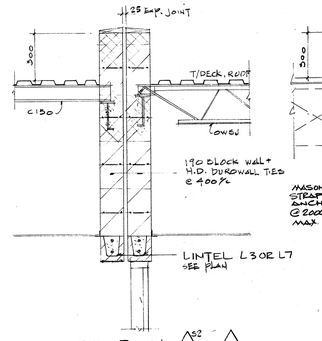
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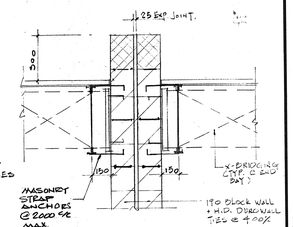
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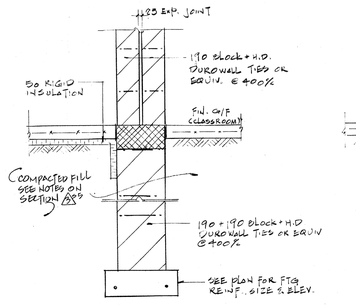
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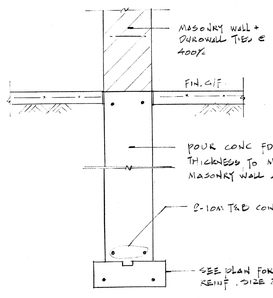
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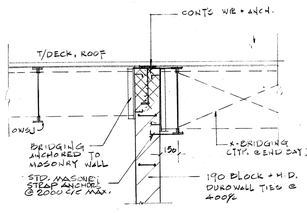
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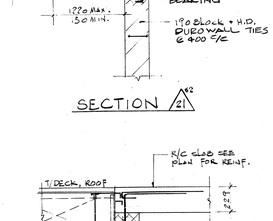
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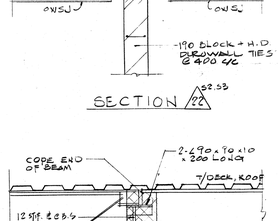
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(ALTERNATIVE FOR MASONRY FOUNDATION WALL)



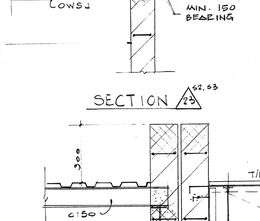
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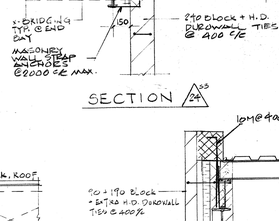
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SECTION 22 S2.53

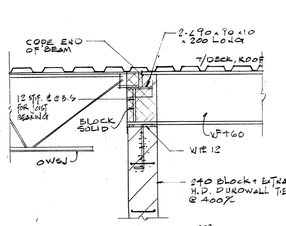


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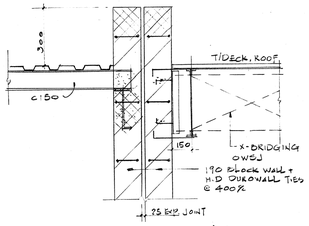


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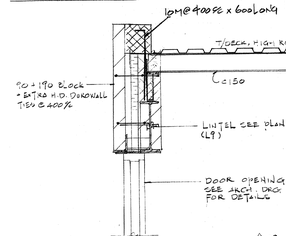
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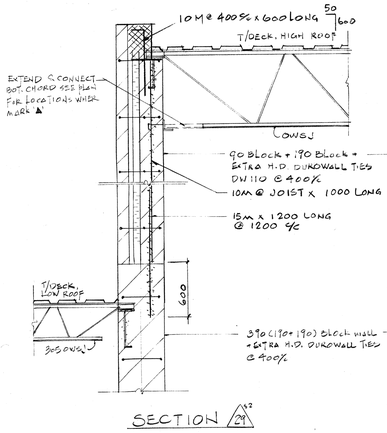
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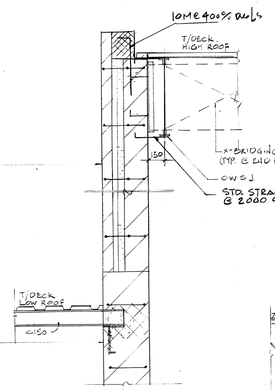
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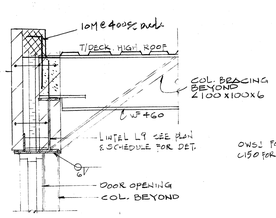
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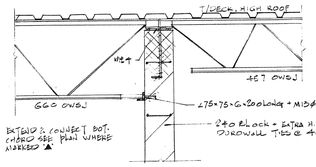
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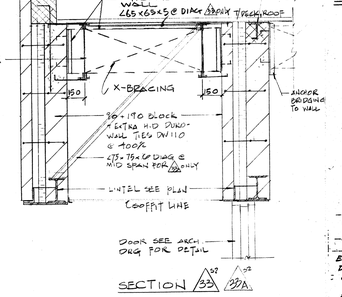
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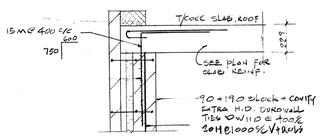
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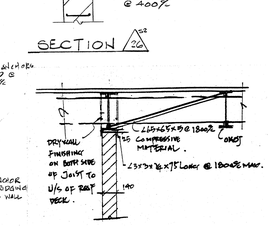
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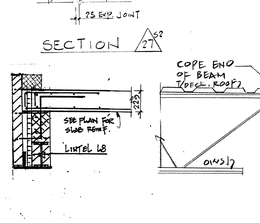
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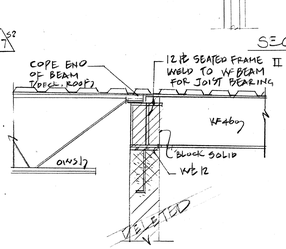
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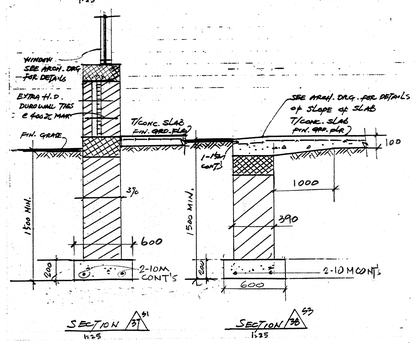
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SECTION 36 S2.53 1:25



SECTION 37 S2



SECTION 38 S3 1:25

SECTION 39 S3 1:25

	FEB. 7 94	ISSUED FOR TENDER
	AUG 18 93	ISSUED FOR PRICING
MARK	DATE	REVISIONS

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190 MARYCROFT AVENUE, UNIT 12,
WOODBRIDGE, ONTARIO, L4L 5Y2
TEL. (416) 851-9595

NOTRE DAME CATHOLIC SCHOOL
FITTONS RD. ORILLIA
SIMCO COUNTY SEPARATE SCHOOL BOARD

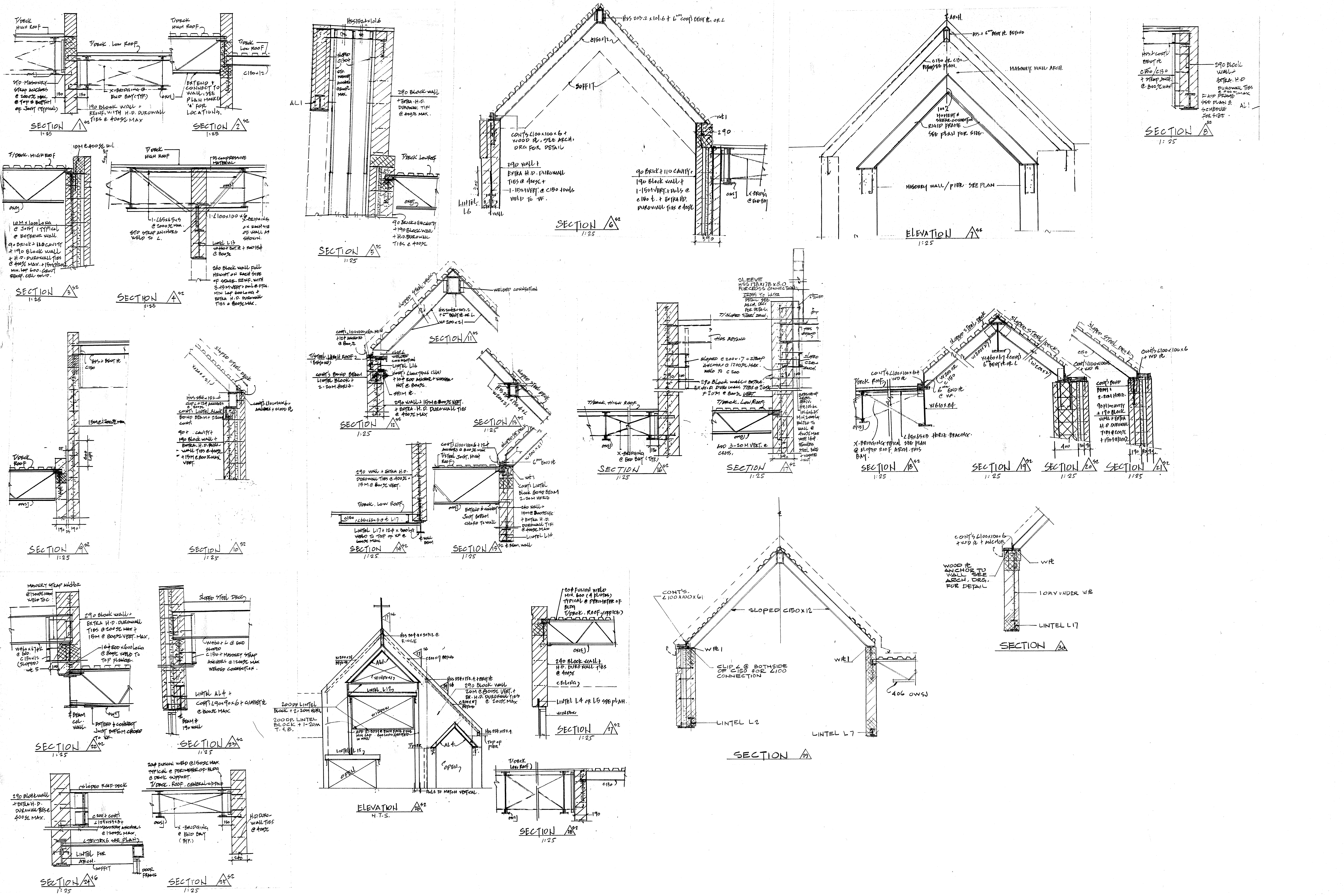
ARCHITECT: R. H. CARTER	
DRAWN BY: B.H.	CHECKED BY: W.L.
DATE: JUNE 1993	SCALE
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ARCHITECT
R. H. CARTER

DRAWN BY	CHECKED BY
B.H.	W.L.
DATE	SCALE
JUNE 1993	
PROJECT NO.	ISSUED DATE
93048	

REGISTERED PROFESSIONAL ENGINEER W. C. K. LEUNG PROVINCE OF ONTARIO 2/7/94

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